# CLOSURE OF INACTIVE NABORS LANDFILL NABORS LANDFILL

1320 RLH LANDFILL ROAD MOUNTAIN HOME, AR 72653

Solid Waste Permit No. 0249-S4 and 0249-S1-R2
ABA Contract No. 4600033394
July 2015

PREPARED FOR

ARKANSAS
Department of Environmental Quality

5301 NORTHSHORE DRIVE NORTH LITTLE ROCK, AR 72118

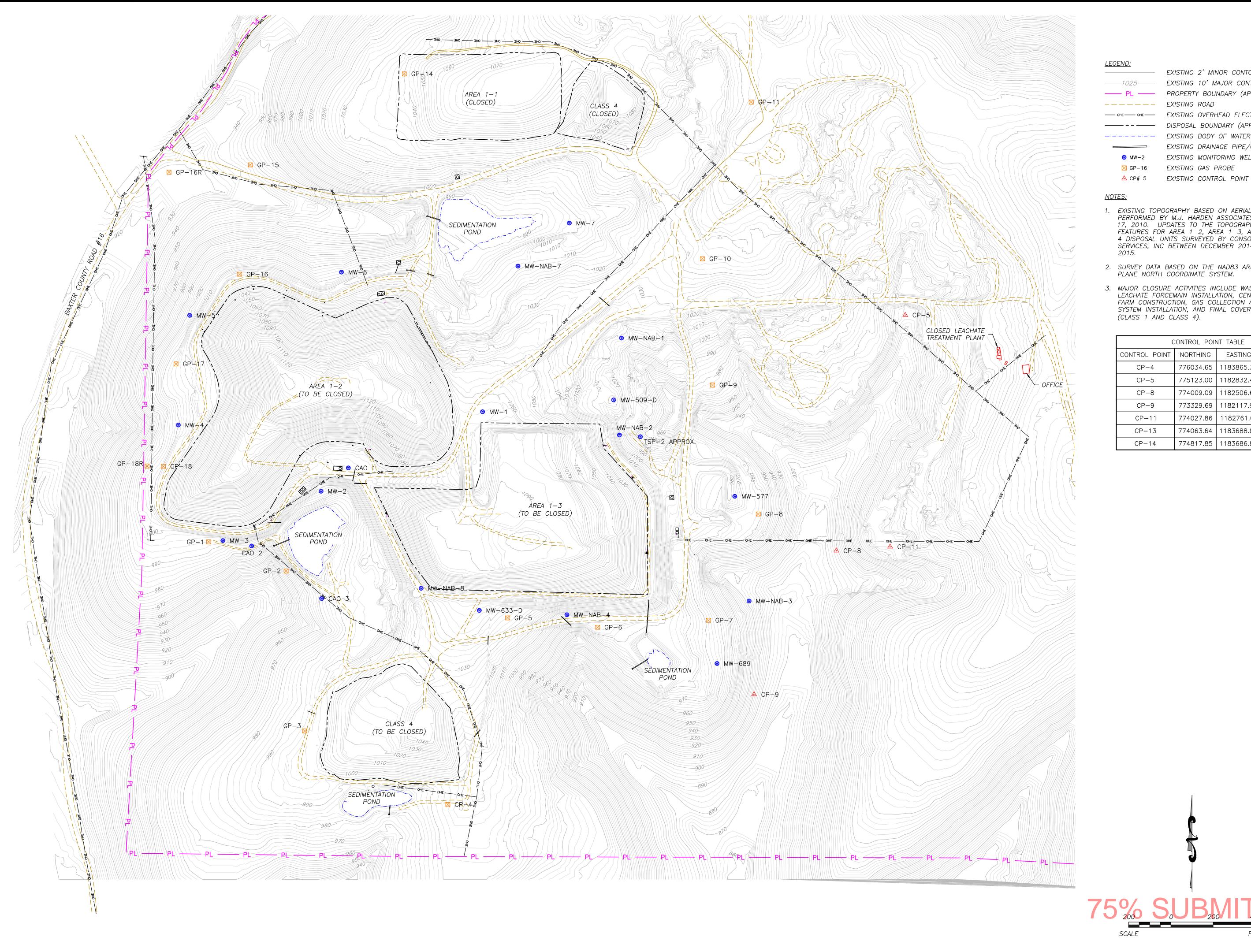
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C-101	Demolition Sheet
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C-302	Area 1-3 Waste Relocation Plan
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C-604	Class 4 Final Grading Plan
C-605	Class 4 Stormwater Control Plan
C-606	Final Cover Details
C-700	Facility Improvements

### PREPARED BY

## SCS AQUATERRA

7311 WEST 130th STREET, SUITE 100 OVERLAND PARK, KANSAS 66213-3117 (913) 681-0030 FAX (913) 681-0012 PROJECT NO. 27214218.01

FOR BIDDING PURPOSES ONLY NOT FOR CONSTRUCTION



<u>LEGEND:</u>	
	EXISTING 2' MINOR CONTOUR
——1025——	EXISTING 10' MAJOR CONTOUR
—— PL ——	PROPERTY BOUNDARY (APPROX.)
	EXISTING ROAD
—— ОНЕ —— ОНЕ ——	EXISTING OVERHEAD ELECTRIC
	DISPOSAL BOUNDARY (APPROXIMAT
	EXISTING BODY OF WATER
	EXISTING DRAINAGE PIPE/CULVERT
	EXISTING MONITORING WELL

1. EXISTING TOPOGRAPHY BASED ON AERIAL SURVEY
PERFORMED BY M.J. HARDEN ASSOCIATES, INC ON AUGUST
17, 2010. UPDATES TO THE TOPOGRAPHY AND SITE
FEATURES FOR AREA 1-2, AREA 1-3, AND SOUTH CLASS
4 DISPOSAL UNITS SURVEYED BY CONSOLIDATED LAND
SERVICES, INC BETWEEN DECEMBER 2014 AND JANUARY
2015.

EXISTING GAS PROBE

- SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.
- 3. MAJOR CLOSURE ACTIVITIES INCLUDE WASTE RELOCATION, LEACHATE FORCEMAIN INSTALLATION, CENTRALIZED TANK FARM CONSTRUCTION, GAS COLLECTION AND CONTROL SYSTEM INSTALLATION, AND FINAL COVER CONSTRUCTION (CLASS 1 AND CLASS 4).

C	CONTROL POI	NT TABLE	
CONTROL POINT	NORTHING	EASTING	ELEVATIO
CP-4	776034.65	1183865.35	1054.93
CP-5	775123.00	1182832.40	1065.34
CP-8	774009.09	1182506.62	1030.49
CP-9	773329.69	1182117.90	991.71
CP-11	774027.86	1182761.01	1039.02
CP-13	774063.64	1183688.83	989.88
CP-14	774817.85	1183686.86	996.14

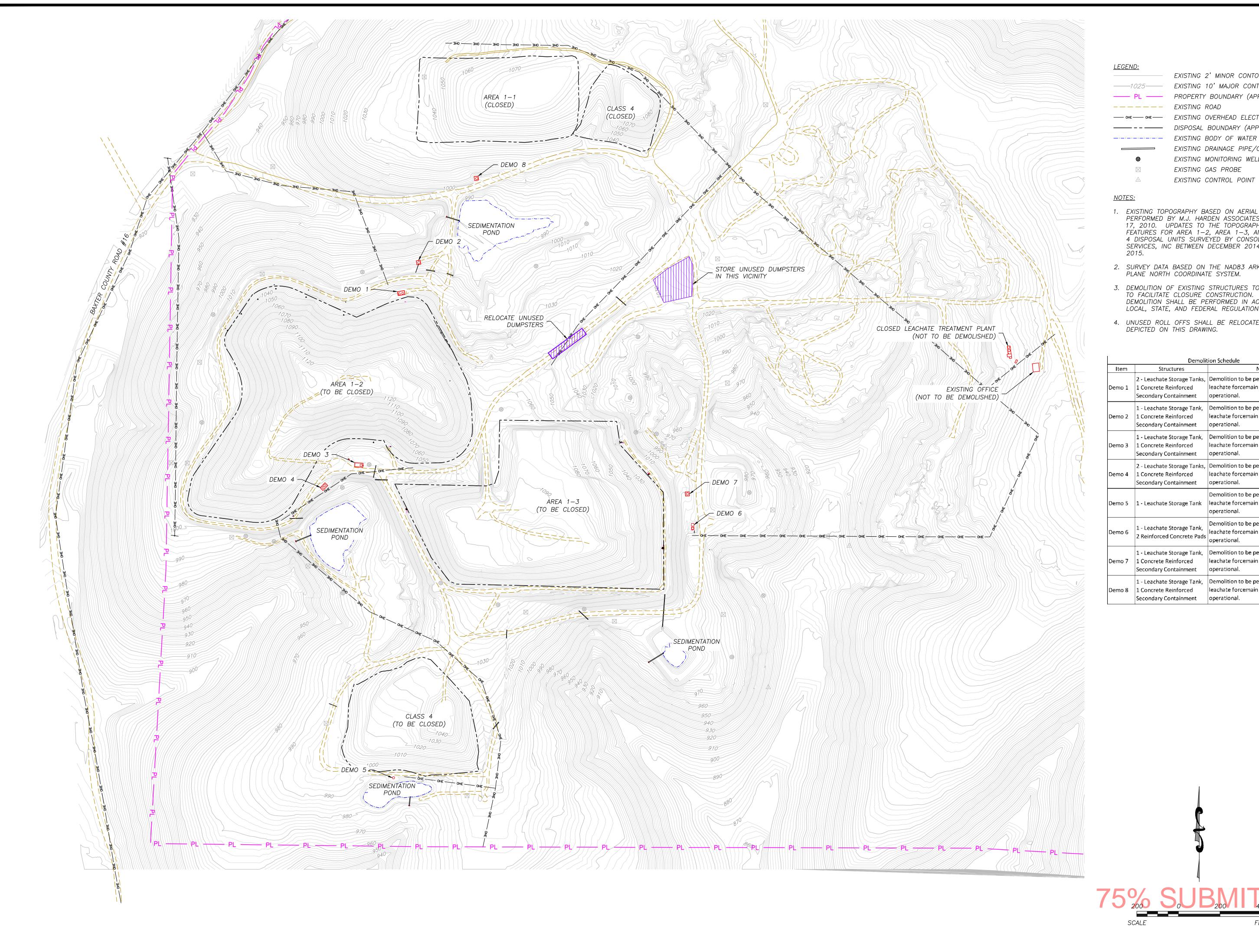
<u> </u>	REV.	REV. DATE	DESCRIPTION	CK.
-				
<b>ABORS LANDFILL</b>	1	7/2015	75% SUBMITTAL	FEC
	0	0 5/2015	50% SUBMITTAL	FEC

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CADD FILE: c-100 - site layout.dwg

7/9/15 DRAWING NO.



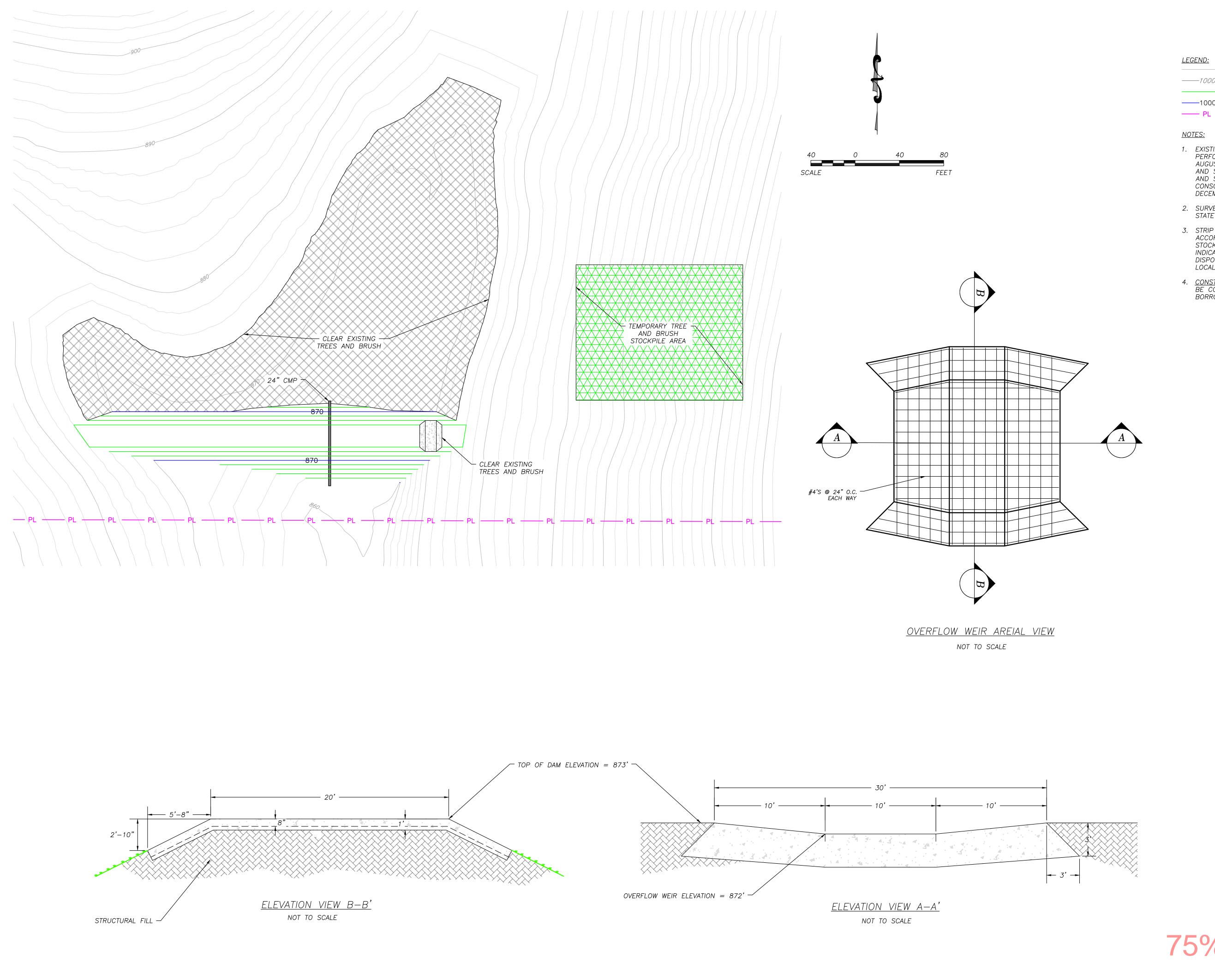
<u>LEGEND:</u>	
	EXISTING 2' MINOR CONTOUR
1 <i>025</i>	EXISTING 10' MAJOR CONTOUR
—— PL ——	PROPERTY BOUNDARY (APPROX.)
	EXISTING ROAD
—— ОНЕ —— ОНЕ ——	EXISTING OVERHEAD ELECTRIC
	DISPOSAL BOUNDARY (APPROXIMATE)
	EXISTING BODY OF WATER
	EXISTING DRAINAGE PIPE/CULVERT
0	EXISTING MONITORING WELL
	EXISTING GAS PROBE
$\wedge$	EXISTING CONTROL POINT

- 1. EXISTING TOPOGRAPHY BASED ON AERIAL SURVEY
  PERFORMED BY M.J. HARDEN ASSOCIATES, INC ON AUGUST
  17, 2010. UPDATES TO THE TOPOGRAPHY AND SITE
  FEATURES FOR AREA 1-2, AREA 1-3, AND SOUTH CLASS
  4 DISPOSAL UNITS SURVEYED BY CONSOLIDATED LAND
  SERVICES, INC BETWEEN DECEMBER 2014 AND JANUARY
- SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.
- J. DEMOLITION OF EXISTING STRUCTURES TO BE PERFORMED TO FACILITATE CLOSURE CONSTRUCTION. DISPOSAL OF THE DEMOLITION SHALL BE PERFORMED IN ACCORDANCE WITH LOCAL, STATE, AND FEDERAL REGULATIONS.
- 4. UNUSED ROLL OFFS SHALL BE RELOCATED TO THE AREAS DEPICTED ON THIS DRAWING.

Item	Structures	Notes
Demo 1	2 - Leachate Storage Tanks, 1 Concrete Reinforced Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 2	1 - Leachate Storage Tank, 1 Concrete Reinforced Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 3	1 - Leachate Storage Tank, 1 Concrete Reinforced Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 4	2 - Leachate Storage Tanks, 1 Concrete Reinforced Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 5	1 - Leachate Storage Tank	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 6	1 - Leachate Storage Tank, 2 Reinforced Concrete Pads	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 7	Leachate Storage Tank,     Concrete Reinforced     Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.
Demo 8	1 - Leachate Storage Tank, 1 Concrete Reinforced Secondary Containment	Demolition to be performed after proposed leachate forcemain is installed and operational.

CADD FILE:
DEMOLITION SHEET.DWG

7/9/15



----- PL ------ PROPERTY BOUNDARY

- 1. EXISTING TOPOGRAPHY BASED ON AERIAL SURVEY PERFORMED BY M.J. HARDEN ASSOCIATES, INC ON AUGUST 17, 2010. UPDATES TO THE TOPOGRAPHY AND SITE FEATURES FOR AREA 1-2, AREA 1-3, AND SOUTH CLASS 4 DISPOSAL UNITS SURVEYED BY CONSOLIDATED LAND SERVICES, INC BETWEEN DECEMBER 2014 AND JANUARY 2015.
- 2. SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.
- 3. STRIP AND CLEAR EXISTING TREES AND BRUSH IN ACCORDANCE WITH THE TECHNICAL SPECIFICATIONS. STOCKPILE STRIPPED VEGETATION AT THE LOCATION INDICATED ON THIS DRAWING PRIOR TO DISPOSAL. DISPOSE OF VEGETATION IN ACCORDANCE WITH LOCAL REGULATIONS.
- 4. <u>CONSTRUCTION SEQUENCE:</u> SEDIMENTATION BASIN TO BE CONSTRUCTED PRIOR TO EXCAVATION OF THE BORROW AREA.

75% SUBMITTAL

REV. DATE DESCRIPTION

1 7/2015 75% SUBMITTAL

0 5/2015 50% SUBMITTAL

LAN VABORS LANDFILL

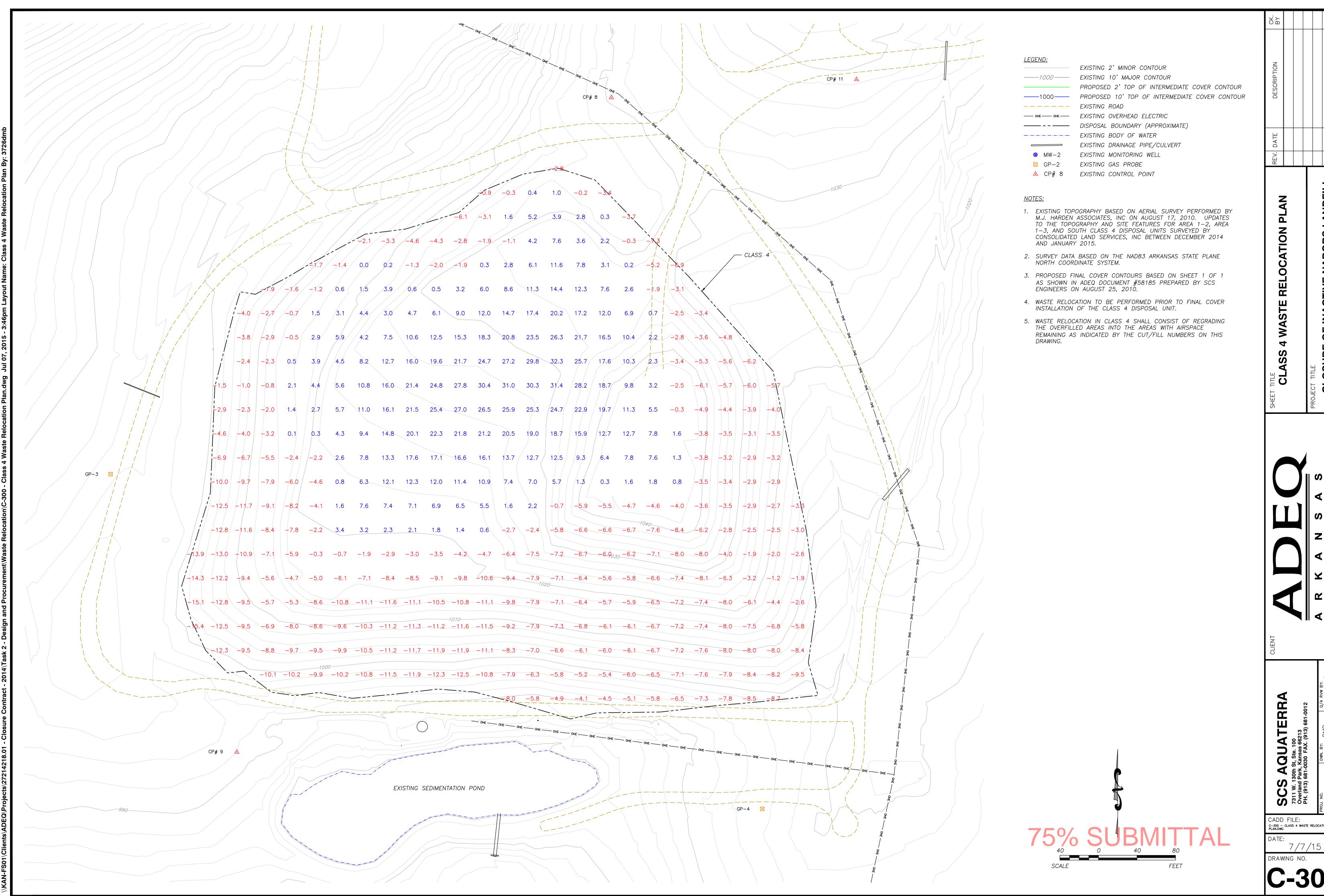
ROPOSED SEDIMENTATIO
GRADING PLAN
TITE
OSURE OF INACTIVE NABOR

A R K A N S A S
Department of Environmental Quality

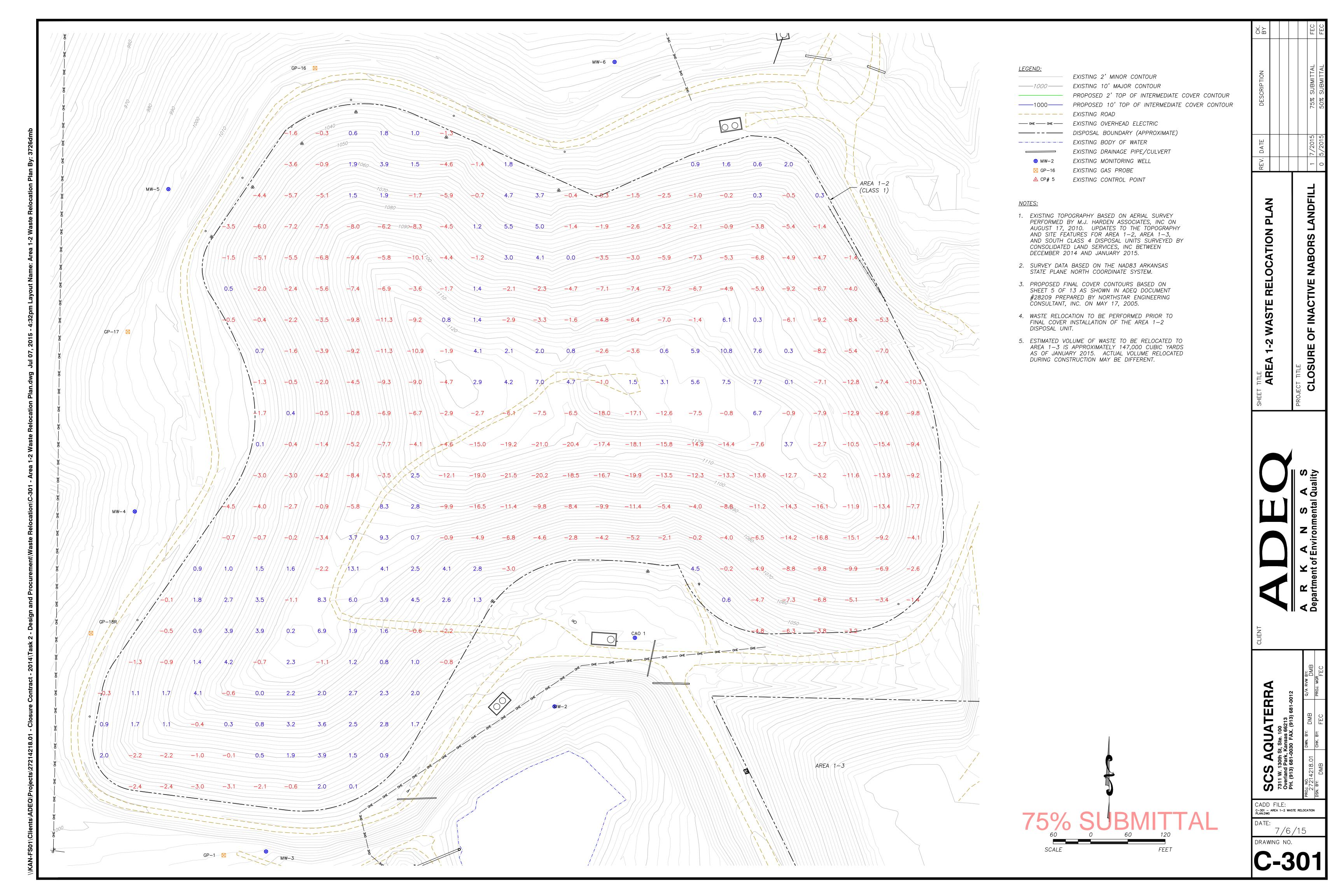
SCS AQUATERRA
7311 W. 130th St, Ste. 100
Diverland Park, Kansas 66213
PH. (913) 681-0030 FAX. (913) 681-0012
1. NO.
1. NO.
27214218.01 DWN. BY: DMB DMB
BY: CHK. BY:

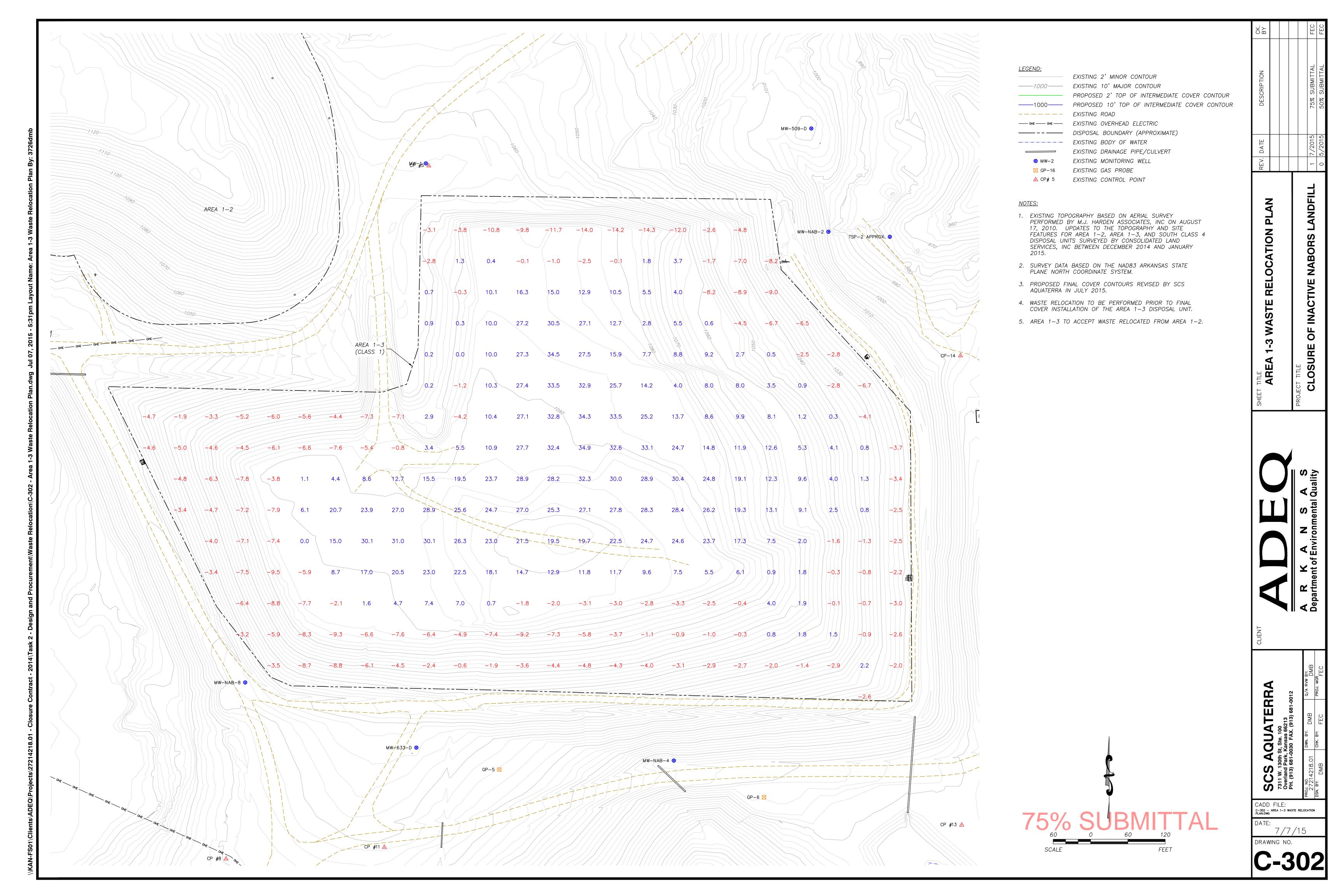
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200 – SEDIMENTATION POND GRADIN
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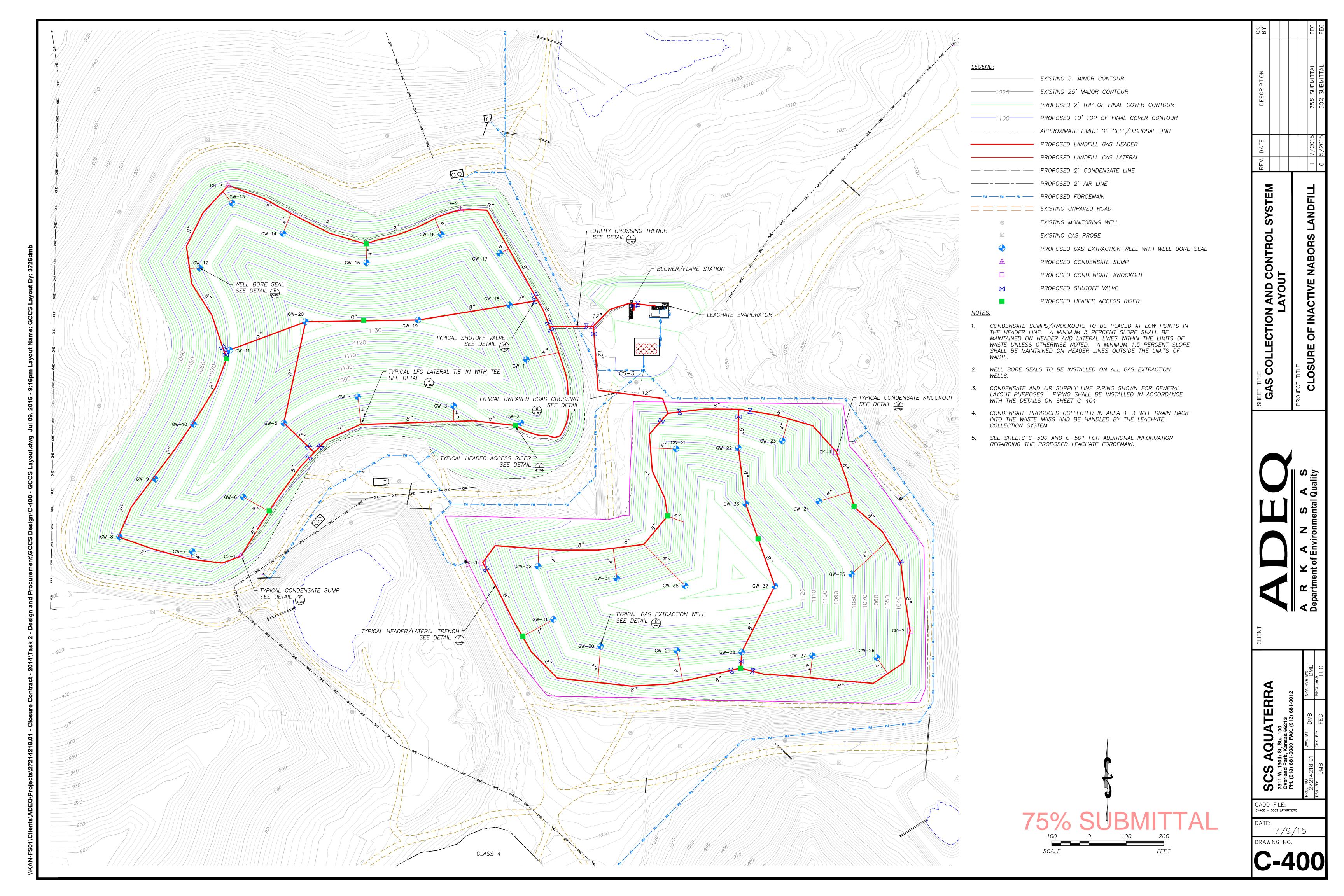
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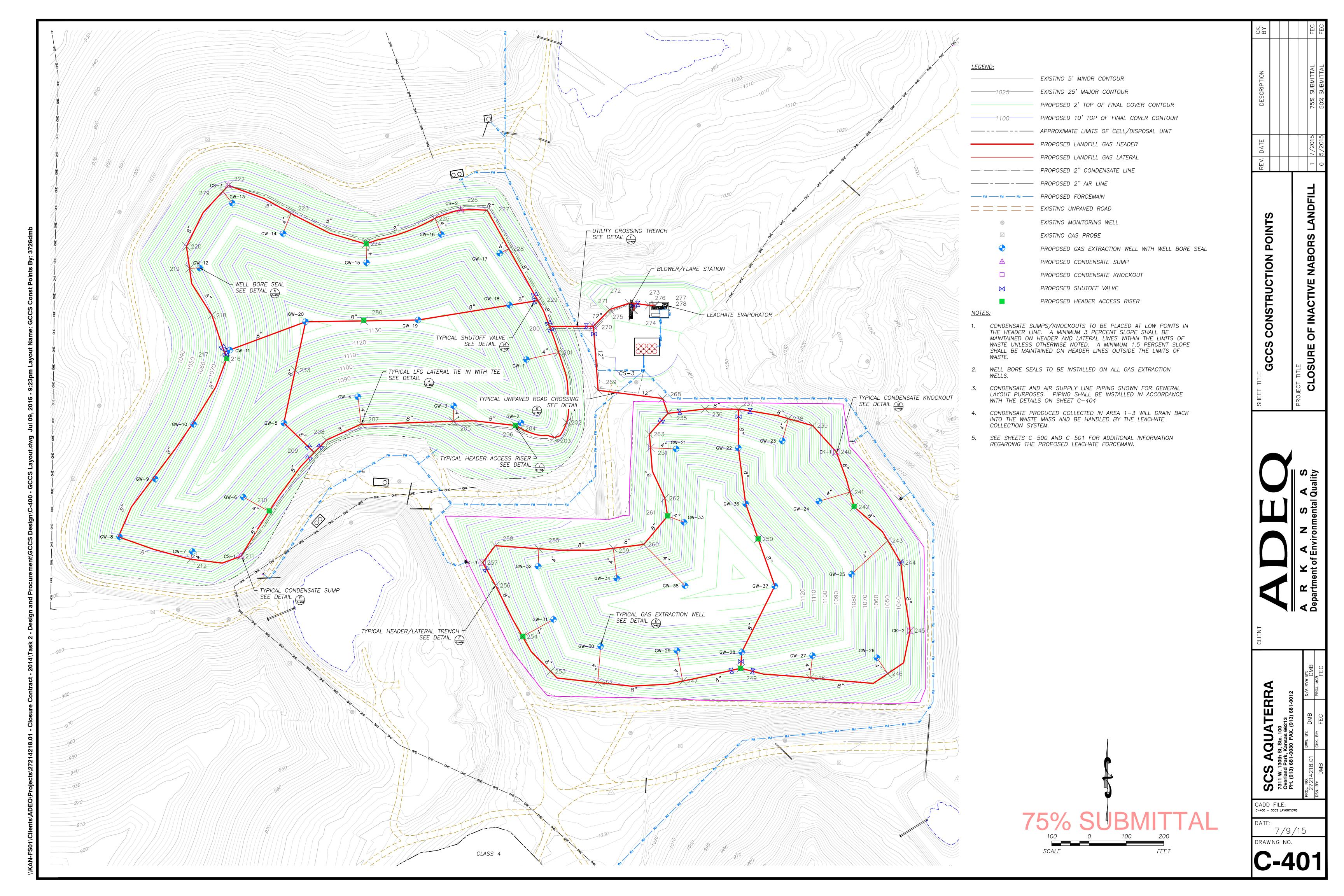


C-300 - CLASS 4 WASTE RELOCATION PLAN.DWG









			Point Table
Point #	Northing	Easting	Description
1	774726.85	1180541.21	GW-1
2	774557.30	1180526.31	GW-2
3	774602.72	1180346.19	GW-3
4	774627.20	1180089.41	GW-4
5	774556.76	1179891.29	GW-5
6	774358.22	1179783.04	GW-6
7	774212.54	1179646.23	GW-7
8	774252.12	1179451.20	GW-8
9	774406.94	1179546.96	GW-9
10	774553.35	1179649.70	GW-10
11	774752.13	1179746.73	GW-11
12	774972.26	1179665.88	GW-12
13	775145.63	1179753.18	GW-13
14	775064.08	1179889.41	GW-14
15	774986.34	1180112.82	GW-15
16	775062.00	1180312.90	GW-16
17	775011.42	1180469.57	GW-17
18	774872.56	1180495.92	GW-18
19	774832.72	1180250.86	GW-19
20	774828.00	1179949.45	GW-20
21	774487.18	1180942.13	GW-21
22	774489.84	1181108.79	GW-22
23	774508.97	1181226.61	GW-23
33	774290.96	1180963.17	GW-33
37	774347.30	1181324.49	GW-24
38	774151.78	1181411.90	GW-25
39	773928.34	1181479.74	GW-26
40	773933.59	1181307.20	GW-27
41	773942.95	1181117.75	GW-28
42	773947.16	1180944.10	GW-29
43	773958.92	1180739.62	GW-30
44	774030.76	1180613.44	GW-31
45	774172.90	1180572.40	GW-32
47	774140.70	1180783.33	GW-34
49	774340.40	1181128.21	GW-36
50	774120.27	1181206.19	GW-37

D : 1 "	N. 11.		D
Point #	Northing	Easting	Description
51	774121.07	1180965.98	GW-38
200	774815.22	1180603.00	Tee: Area 1—2 Header to Flare
201	774745.77	1180627.53	Tee: Area 1—2 Header with Lateral
202	774558.48	1180648.66	Area 1—2 Header
203	774517.96	1180616.77	Area 1-2 Header
204	774542.22	1180526.03	Tee: Area 1—2 Header with Lateral
205	774561.28	1180345.55	Tee: Area 1—2 Header with Lateral
206	774550.23	1180512.01	Area 1—2 Header: Access Riser
207	774553.03	1180098.33	Tee: Area 1—2 Header with Lateral
208	774510.99	1180005.15	Area 1-2 Header
209	774477.72	1179972.74	Tee: Area 1—2 Header to Second Jumper
210	774319.96	1179851.72	Tee: Area 1—2 Header with Lateral and Access Ris
211	774199.97	1179776.31	Area 1—2 Header: Condensate Sump 1
212	774191.91	1179642.05	Tee: Area 1—2 Header with Lateral
216	774728.88	1179737.77	Area 1-2 Header: Access Riser
217	774746.56	1179731.84	Tee: Area 1—2 Header to Main Jumper
218	774845.48	1179698.64	Area 1-2 Header
219	774971.01	1179638.66	Tee: Area 1—2 Header with Lateral
220	775029.87	1179630.40	Area 1-2 Header
222	775192.94	1179742.77	Area 1—2 Header: Condensate Sump 3
223	775118.32	1179910.61	Tee: Area 1—2 Header with Lateral
224	775036.78	1180111.93	Tee: Area 1—2 Header Lateral and Access Riser
225	775104.42	1180294.77	Tee: Area 1—2 Header with Lateral
226	775127.32	1180364.59	Area 1—2 Header: Condensate Sump 2
227	775126.96	1180434.33	Area 1-2 Header
228	775023.37	1180493.77	Tee: Area 1—2 Header with Lateral
229	774883.46	1180572.29	Tee: Area 1—2 Header to Main Jumper
233	774698.68	1179921.72	Area 1—2 Second Jumper
235	774582.80	1180919.99	Tee: Area 1—3 Header to Flare
236	774594.51	1181019.49	Area 1-3 Header
237	774593.79	1181109.13	Tee: Area 1—3 Header to Jumper
238	774568.48	1181242.50	Tee: Area 1—3 Header with Lateral
239	774549.67	1181308.05	Area 1-3 Header
240	774479.41	1181371.05	Area 1—3 Header: Condensate Knockout 1
241	774371.74	1181407.75	Tee: Area 1—3 Header with Lateral
242	774332.34	1181418.97	Area 1-3 Header: Access Riser

			Point Table
Point #	Northing	Easting	Description
243	774241.52	1181508.91	Tee: Area 1—3 Header with Lateral
244	774183.01	1181543.68	Area 1—3 Header
245	774000.77	1181568.74	Area 1—3 Header: Condensate Knockout 2
246	773885.65	1181508.82	Tee: Area 1—3 Header with Lateral
247	773865.49	1180959.64	Tee: Area 1—3 Header with Lateral
248	773874.49	1181300.37	Tee: Area 1—3 Header with Lateral
249	773899.97	1181114.91	Tee: Area 1—3 Header to Jumper and Access Riser
250	774246.24	1181161.36	Area 1—3 Jumper: Access Riser
251	774490.50	1180873.38	Tee: Area 1—3 Header with Lateral
252	773861.78	1180732.03	Tee: Area 1—3 Header with Lateral
253	773891.63	1180605.78	Area 1—3 Header
254	773984.97	1180530.62	Tee: Area 1—3 Header with Lateral and Access Riser
255	774220.20	1180574.03	Tee: Area 1—3 Header with Lateral
256	774121.83	1180458.23	Area 1—3 Header
257	774182.87	1180424.09	Area 1—3 Header Condensate Knockout 3
258	774228.32	1180457.30	Area 1—3 Header
259	774221.39	1180773.47	Tee: Area 1—3 Header with Lateral
260	774231.56	1180855.47	Tee: Area 1—3 Header with Lateral
261	774307.64	1180919.10	Tee: Area 1—3 Header with Lateral and Access Riser
262	774355.36	1180911.72	Area 1—3 Header
263	774527.12	1180870.50	Area 1—3 Header
268	774622.59	1180905.81	Flare Input
269	774644.51	1180733.09	Flare Input
270	774816.00	1180720.47	Flare Input Condensate Sump
271	774860.74	1180764.03	Flare Input
272	774877.62	1180816.09	Flare Input
273	774877.61	1180829.31	Tee: Flare Input or Evaporator
274	774859.34	1180829.23	Flare Input
275	774859.34	1180825.47	Flare Connection
276	774872.16	1180869.44	Evaporator Input
277	774872.16	1180885.17	Evaporator Input
278	774869.37	1180885.16	Evaporator Connection
279	775177.23	1179728.01	Tee: Area 1—3 Header with Lateral
280	774830.83	1180105.35	Area 1—2 Jumper: Access Riser

1. SEE WELL SCHEDULE FOR ADDITIONAL GAS EXTRACTION WELL DRILLING INFORMATION.

75% SUBMITTAL

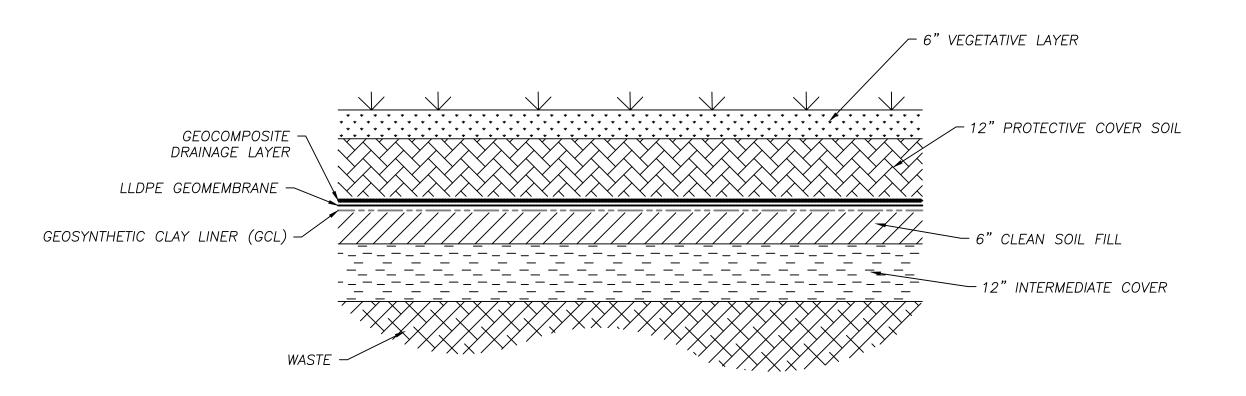
CADD FILE: c-400 - gccs layout.dwg

DATE: 7/9/15

Vertical  Extraction  Well	AR State Pla  North Zone		Proposed	Existing	Difference	Top Of	Required					
Extraction Well	North Zone	e					110 quii ou					
Well			Final	Surface	In	Base Liner	Depth Above	Available	Proposed	8" PVC	8" CPVC	8" PVC
			Grade	Grade	Surface	Grade	Base Liner	Drilling	Drilling	Under Ground	Perforated	Above Ground
- · ·	Coordinates	Coordinates	Elevation	Elevation	Elevation	Elevation	System	Depth	Depth	Solid Pipe	Pipe	Solid Pipe
Designation	North	East	(MSL)	(MSL)	(FT.)	(MSL)	(FT.)	(FT.)	(FT.)	(FT.)	(FT.)	Pipe (FT.)
GW-1	774,726.85	1,180,541.21	1,098.54	1,096.54	2.00	1,059.42	15	22.0	22.0	10.0	11.0	4.0
GW-2	774,557.30	1,180,526.31	1,089.84	1,087.84	2.00	1,053.01	15	20.0	20.0	10.0	9.0	4.0
GW-3	774,602.72	1,180,346.19	1,082.07	1,080.07	2.00	1,047.89	15	17.0	17.0	10.0	6.0	4.0
GW-4	774,627.20	1,180,089.41	1,078.23	1,076.23	2.00	1,038.06	15	23.0	23.0	10.0	12.0	4.0
GW-5	774,556.76	1,179,891.29	1,082.39	1,080.39	2.00	1,021.53	15	44.0	44.0	16.0	27.0	4.0
GW-6	774,358.22	1,179,783.04	1,066.23	1,064.23	2.00	1,004.19	15	45.0	45.0	16.0	28.0	4.0
GW-7	774,212.54	1,179,646.23	1,045.16	1,043.16	2.00	995.00	15	33.0	33.0	16.0	16.0	4.0
GW-8	774,252.12	1,179,451.20	1,048.09	1,046.09	2.00	999.67	15	31.0	31.0	10.0	20.0	4.0
GW-9	774,406.94	1,179,546.96	1,065.47	1,063.47	2.00	1,006.66	15	42.0	42.0	16.0	25.0	4.0
GW-10	774,553.35	1,179,649.70	1,071.84	1,069.84	2.00	1,019.95	15	35.0	35.0	16.0	18.0	4.0
GW-11	774,752.13	1,179,746.73	1,082.01	1,080.01	2.00	1,033.76	15	31.0	31.0	10.0	20.0	4.0
GW-12	774,972.26	1,179,665.88	1,078.70	1,076.70	2.00	1,032.59	15	29.0	29.0	10.0	18.0	4.0
GW-13	775,145.63	1,179,753.18	1,070.18	1,068.18	2.00	1,029.48	15	24.0	24.0	10.0	13.0	4.0
GW-14	775,064.08	1,179,889.41	1,080.90	1,078.90	2.00	1,031.93	15	32.0	32.0	16.0	15.0	4.0
GW-15	774,986.34	1,180,112.82	1,082.70	1,080.70	2.00	1,038.41	15	27.0	27.0	16.0	10.0	4.0
GW-16	775,062.00	1,180,312.90	1,077.55	1,075.55	2.00	1,040.86	15	20.0	20.0	10.0	9.0	4.0
GW-17	775,011.42	1,180,469.57	1,074.94	1,072.94	2.00	1,039.92	15	18.0	18.0	10.0	7.0	4.0
GW-18	774,872.56	1,180,495.92	1,088.24	1,086.24	2.00	1,048.51	15	23.0	23.0	10.0	12.0	4.0
GW-19	774,832.72	1,180,250.86	1,133.77	1,131.77	2.00	1,046.00	15	71.0	71.0	16.0	54.0	4.0
GW-20	774,828.00	1,179,949.45	1,134.38	1,132.38	2.00	1,037.91	15	79.0	79.0	16.0	62.0	4.0
GW-21	774,487.18	1,180,942.13	1,096.54	1,094.54	2.00	1,059.96	15	20.0	20.0	10.0	9.0	4.0
GW-22	774,489.84	1,181,108.79	1,087.23	1,085.23	2.00	1,054.00	15	16.0	16.0	10.0	5.0	4.0
GW-23	774,508.97	1,181,226.61	1,074.00	1,072.00	2.00	1,040.05	15	17.0	17.0	10.0	6.0	4.0
GW-24	774,347.30	1,181,324.49	1,067.17	1,065.17	2.00	1,029.63	15	21.0	21.0	10.0	10.0	4.0
GW-25	774,151.78	1,181,411.90	1,081.65	1,079.65	2.00	1,020.65	15	44.0	44.0	16.0	27.0	4.0
GW-26	773,928.34	1,181,479.74	1,059.85	1,057.85	2.00	1,012.00	15	31.0	31.0	10.0	20.0	4.0
GW-27	773,933.59	1,181,307.20	1,081.72	1,079.72	2.00	1,029.18	15	36.0	36.0	16.0	19.0	4.0
GW-28	773,942.95	1,181,117.75	1,090.67	1,088.67	2.00	1,049.33	15	24.0	24.0	10.0	13.0	4.0
GW-29	773,947.16	1,180,944.10	1,097.32	1,095.32	2.00	1,057.05	15	23.0	23.0	10.0	12.0	4.0
GW-30	773,958.92	1,180,739.62	1,092.75	1,090.75	2.00	1,054.88	15	21.0	21.0	10.0	10.0	4.0
GW-31	774,030.76	1,180,613.44	1,082.51	1,080.51	2.00	1,048.80	15	17.0	17.0	10.0	6.0	4.0
GW-32	774,172.90	1,180,572.40	1,084.00	1,082.00	2.00	1,044.83	10	27.0	27.0	10.0	16.0	4.0
GW-34	774,140.70	1,180,783.33	1,108.72	1,106.72	2.00	1,055.93	15	36.0	36.0	16.0	19.0	4.0
GW-35	774,290.96	1,180,963.17	1,110.40	1,108.40	2.00	1,058.65	15	35.0	35.0	16.0	18.0	4.0
GW-36	774,340.40	1,181,128.21	1,111.35	1,109.35	2.00	1,054.42	15	40.0	40.0	16.0	23.0	4.0
GW-37	774,120.27	1,181,206.19	1,125.98	1,123.98	2.00	1,043.64	15	65.0	65.0	16.0	48.0	4.0
GW-38	774,121.07	1,180,965.98	1,124.51	1,122.51	2.00	1,058.14	15	49.0	49.0	16.0	32.0	4.0

#### <u>NOTES:</u>

- 1. GAS EXTRACTION WELL COORDINATES AND SURFACE ELEVATIONS, UNLESS NOTED OTHERWISE, BASED ON THE PROPOSED FINAL COVER DESIGNS.
- 2. AREA 1—2 AND AREA 1—3 LINER ELEVATIONS BASED ON AS—BUILT DOCUMENTATION, WHERE AVAILABLE, PROVIDED BY OTHERS. DRILL DEPTHS DEVELOPED BASED ON COMPILED LINER ELEVATIONS.
- 3. WELL BORE SEALS TO BE INSTALLED ON WELLS WITH LESS THAN 10 FEET OF SOLID PIPE BELOW TOP OF FINAL COVER ELEVATION.
- 4. COORDINATES AND SURFACE ELEVATIONS HAVE YET TO BE VERIFIED WITH A FIELD SURVEY. THESE VALUES WILL BE REVISED AND APPROVED BY THE ENGINEER, PRIOR TO THE COMMENCEMENT OF DRILLING.



#### FINAL COVER NOTES:

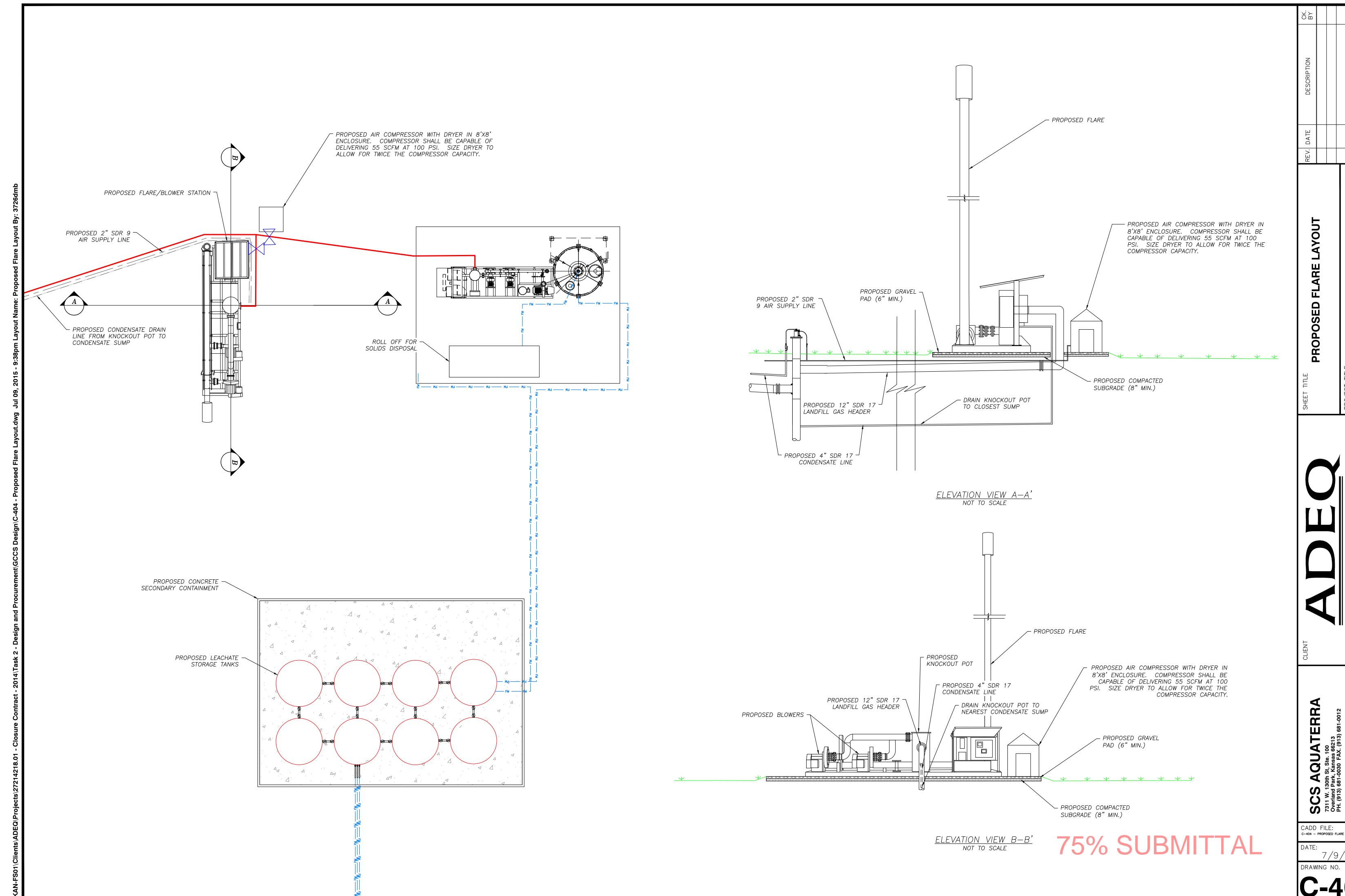
- 1. REPAIR FINAL COVER TO MATCH EXISTING CONDITIONS.
- 2. LOW PERMEABILITY COVER LAYER TO BE COMPACTED TO 90 PERCENT STANDARD PROCTOR AND BETWEEN 0% TO +2.5% OPTIMUM MOISTURE CONTENT OR WITHIN PLACEMENT ZONE. PLACEMENT ZONE TO BE PROVIDED UPON REQUEST.

1 CLASS 1 COMPOSITE FINAL COVER CROSS SECTION

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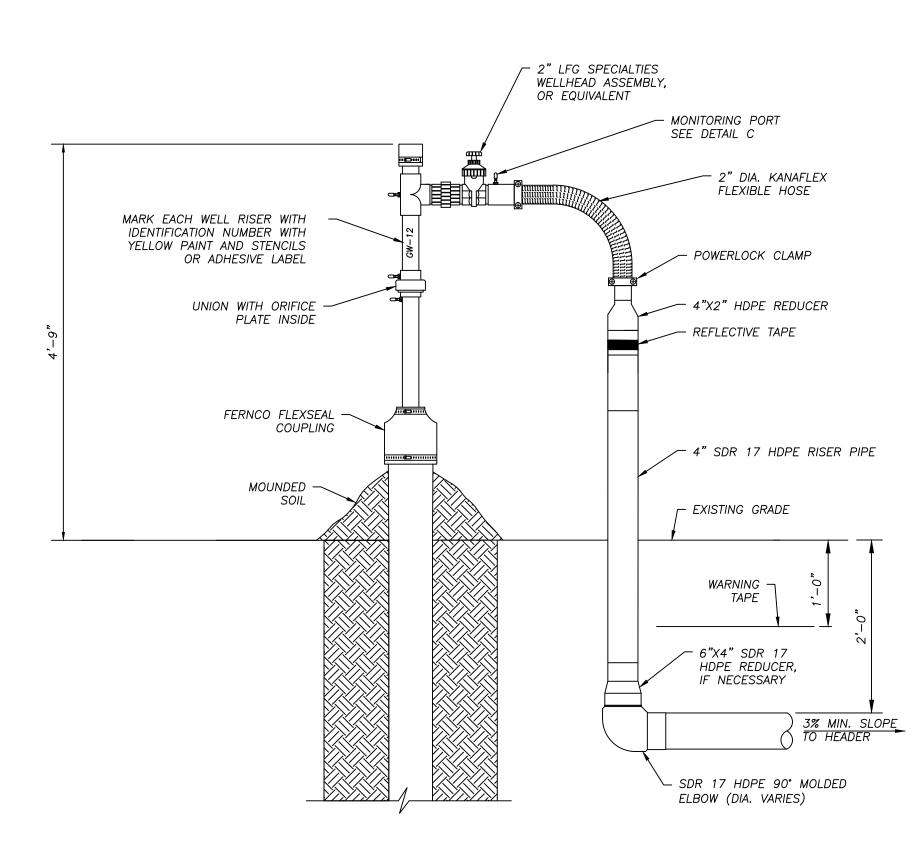
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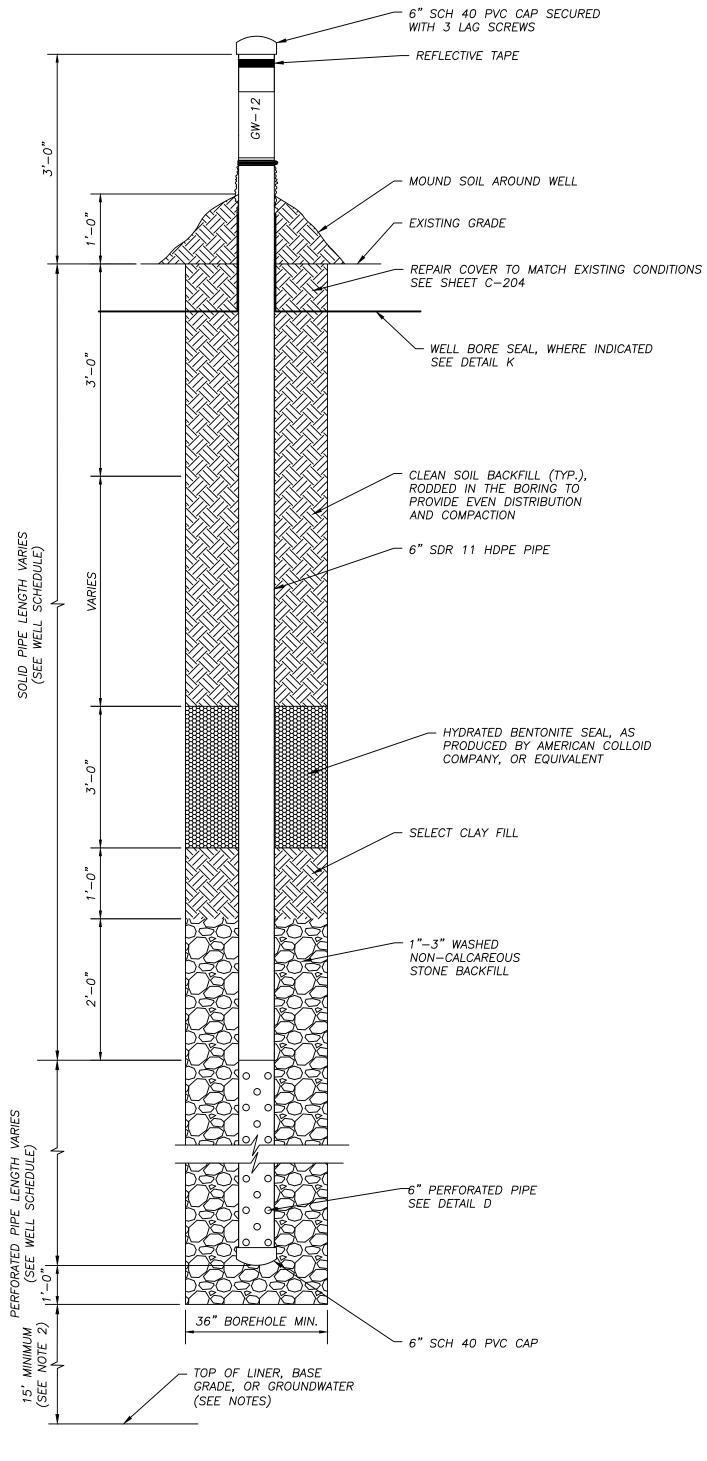
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#### <u>NOT</u>

- 1. WELLHEAD ASSEMBLIES SHALL BE SUPPLIED AS PREFABRICATED AND SHOP TESTED ASSEMBLIES.
- 2. WELLHEAD PIPING AND FITTINGS SHALL BE SCHEDULE 80 PVC.
- 3. WELLHEAD ASSEMBLY UNITS SHALL BE SUITABLE FOR MEASURING LANDFILL GAS FLOW UP TO 50 CUBIC FEET PER MINUTE AND SHALL INCORPORATE BUILT—IN MEASUREMENT TUBES, IMPACT TUBING, UNIONS, FITTINGS, TEMPERATURE GAUGES OR PORTS, QUICK CONNECT PORTS, AND NUTS, BOLTS, AND GASKETS.
- 4. NUTS, BOLTS, AND GASKETS SHALL BE CADMIUM PLATED, GALVANIZED STEEL, STAINLESS STEEL, OR ZINC PLATE FOR ABOVEGROUND INSTALLATION AND STAINLESS STEEL FOR BELOW GROUND INSTALLATION.
- 5. KANAFLEX HOSE SHALL BE INSTALLED TO PROVIDE 50 PERCENT CONTRACTION, 20 PERCENT EXTENSION, AND SUFFICIENT SLACK TO ALLOW FOR PIPING EXPANSION AND CONTRACTION WITHOUT CREATION OF LOW POINTS IN THE LINE.





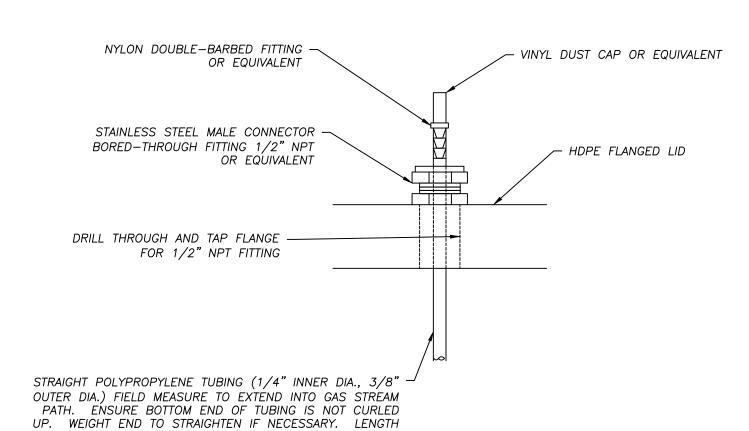
#### <u>NOTES</u>

- 1. ENGINEER SHALL ADJUST WELL SCHEDULE AS NECESSARY BASED ON THE PRE-CONSTRUCTION SURVEY.
- 2. TOTAL DRILL DEPTHS WILL BE A MINIMUM 15' ABOVE THE TOP OF LOW PERMEABILITY LINER ELEVATIONS.
- 3. THE WASHED NON-CALCEROUS STONE BACKFILL SHALL BE FREE FROM DIRT, VEGETATION, OR OTHER OBJECTIONABLE MATTER, AND FREE FROM AN EXCESS OF SOFT, THIN ELONGATED, LAMINATED OR DISINTEGRATED PIECES.
- 4. UNDER NO CIRCUMSTANCE SHALL THE BOREHOLE DRILLING CONTINUE BELOW THE WELL DEPTH ELEVATION IDENTIFIED IN THE WELL SCHEDULE.

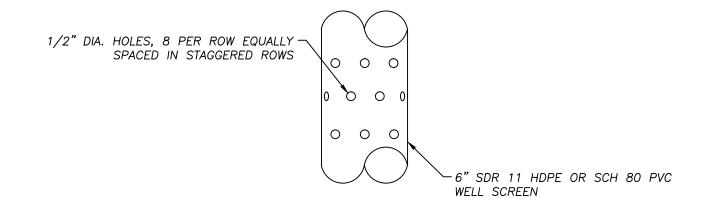


#### **GENERAL NOTES:**

- 1. EXCAVATED REFUSE/DRILL CUTTINGS FROM THE CONSTRUCTION AREAS SHALL BE IMMEDIATELY DISPOSED OF AT THE ACTIVE FACE OF AREA  $1\!-\!3$ .
- 2. IN THE EVENT THAT A BOREHOLE MUST BE ABANDONED, THE INSTALLER SHALL PLUG AND ABANDON THE HOLE FROM THE BOTTOM TO WITHIN FIVE FEET OF THE SURFACE USING SOIL AND FROM FIVE FEET BELOW THE SURFACE TO THE SURFACE USING A CEMENT GROUT OR GRADED BENTONITE PLUGGING MATERIAL IN A MANNER APPROVED BY THE CQA ENGINEER OR TECHNICIAN.
- 3. A SAFETY GRATE THAT PROVIDES AT LEAST 2 FEET OF OVERLAP OVER THE BOREHOLE CORNERS OR EDGES AND IS DESIGNED AND CONSTRUCTED TO HOLD AT LEAST TWICE THE WEIGHT OF THE MATERIALS AND EQUIPMENT (INCLUDING PIPING CLAMPED TO THE CENTER OF THE GRATE) PLACED ON THE GRATE WILL BE AVAILABLE AT ALL TIMES DURING DRILLING ACTIVITIES. THE GRATE SHOULD BE USED TO COVER ANY OPEN BOREHOLES WHEN PERSONS ARE WITHIN THE CONTROLLED ACCESS ZONE.
- 4. NO BACKFILLING OPERATIONS SHALL BE PERMITTED UNTIL THE BENTONITE HAS BEEN HYDRATED. HYDRATION FOR BENTONITE PLUG IS A MINIMUM 30 MINUTES OR AS RECOMMENDED BY THE MANUFACTURER.
- 5. COMBUSTIBLE LANDFILL GAS IS EXPECTED TO VENT FROM BOREHOLES AND SHALL BE MONITORED AND CONTROLLED IN SUCH A MANNER AS TO SAFELY CONSTRUCT THE WELLS, PREVENT VIOLATION OF ALL APPLICABLE AIR QUALITY REGULATIONS, AND PREVENT WORKER EXPOSURE
- 6. A WATER TRUCK WITH SUITABLE SPRAYING EQUIPMENT SHALL BE KEPT ON SITE AT ALL TIMES DURING DRILLING. DURING DRILLING INTO DRY REFUSE, WATER WILL BE PERIODICALLY SPRAYED INTO BOREHOLES JUST SUFFICIENT TO DAMPEN THE CUTTINGS.
- 7. PIPES AND FITTINGS SHALL BE CONSTRUCTED OF HDPE TYPE PE 4710 RESIN.







#### <u>NOTES</u>:

OF TUBING SHALL BE APPROVED BY ENGINEER BASED ON

THE INSTALLED DEPTH OF HEADER

- 1. PERFORATIONS SPACED 90° APART HORIZONTALLY.
- 2. PERFORATIONS SPACED 4" APART VERTICALLY.
- 3. 90° AND 270° ROWS STAGGERED 2" BELOW 0° AND 180° ROWS.



75% SUBMITTAL

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_	1	7/2015	75% SUBMITTAL	34
	0	0 5/2015	50% SUBMITTAL	FE

GCCS DETAILS 1

TILE

OSURE OF INACTIVE NABORS LANDFILL

A N S A S Environmental Quality

A R K A N
Department of Environm

100 s 66213 AX. (913) 681-0012 3Y: ZDB Q/A RVW BY:

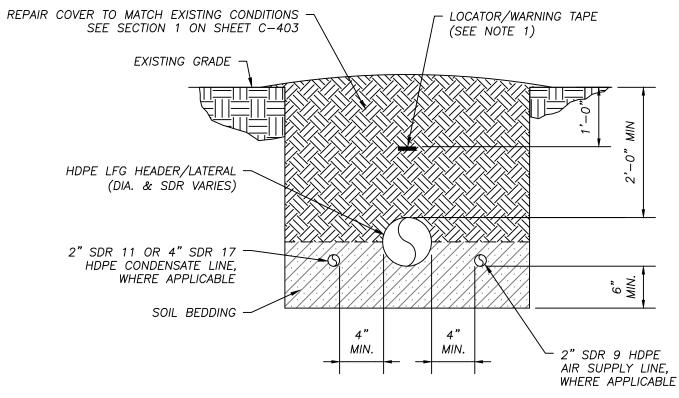
SCS AQUAT 7311 W. 130th St, Ste. 100 Overland Park, Kansas 6621; PH. (913) 681-0030 FAX. (91

CADD FILE: c-405 - DETAILS.DWG

DATE: 7/9/15

DRAWING NO.

- 1. EXCAVATED MATERIALS FROM THE CONSTRUCTION AREA MAY BE SEGREGATED AND STOCKPILED FOR REUSE AS FILL MATERIAL AS DIRECTED BY THE CQA TECHNICIAN. UNUSED EXCAVATED FILL MATERIALS SHALL BE DISPOSED AT THE SITE UNDER THE DIRECTION OF THE OWNER OR CQA TECHNICIAN.
- BACKFILL MATERIALS SHALL BE REASONABLY WELL—GRADED SOILS FREE OF BROKEN CONCRETE AND PAVEMENT, WOOD, ORGANIC MATERIALS, REFUSE, OR OTHER DELETERIOUS MATERIALS.
- 3. NO BACKFILL MATERIAL SHALL BE LARGER THAN FOUR INCHES IN SIZE.
- 4. THE LENGTH OF TRENCH EXCAVATION SHALL BE LIMITED TO THE INSTALLER'S CAPABILITIES TO EXCAVATE, INSTALL THE PIPE, BACKFILL, AND COMPACT THE TRENCH IN ONE WORKING DAY. NO TRENCH SHALL BE ALLOWED TO REMAIN OPEN OVERNIGHT, UNLESS OTHERWISE APPROVED BY THE CQA TECHNICIAN.
- 5. TRENCH EXCAVATIONS WHICH EXCEED FOUR FEET IN DEPTH SHALL COMPLY WITH THE APPLICABLE TRENCH SAFETY STANDARDS AS STATED IN THE OSHA EXCAVATION SAFETY STANDARDS 29 CFR 1926.650 SUBPART P AND LANDFILL GAS DIVISION OF THE SOLID WASTE ASSOCIATION OF NORTH AMERICA.
- 6. THE PIPE SHALL BE LAID IN A FLAT—BOTTOM TRENCH WHICH HAS BEEN CAREFULLY GRADED AND SHAPED WITH BEDDING IN PLACE SO THAT THE BARREL OF THE PIPE WILL HAVE BEARING FOR ITS FULL LENGTH. BLOCKING OF THE PIPE SHALL NOT BE PERMITTED.
- 7. FIELD SURVEY PORTS/WITNESS RISERS SHALL BE CONSTRUCTED AS DIRECTED BY THE CQA TECHNICIAN. VERTICAL PVC PIPES SHALL BE INSTALLED IN TRENCHES EXTENDING FROM THE GROUND SURFACE TO THE TOP OF THE BURIED PIPING EVERY 50 FEET, AT FITTINGS, AND AT CHANGES IN PIPELINE GRADE AND ALIGNMENT. BACKFILL AROUND THE SURVEY PORTS AS DIRECTED BY THE CQA TECHNICIAN. A PVC CAP SHALL BE PLACED ON TOP OF THE SURVEY PORTS.
- 8. EXCAVATIONS SHALL BE BACKFILLED TO THE ORIGINAL GRADES UNLESS OTHERWISE SHOWN ON THE CONSTRUCTION ISSUE DRAWINGS. DEVIATIONS FROM THESE GRADES DUE TO SETTLING SHALL BE CORRECTED BY THE INSTALLER AS DIRECTED BY THE CQA TECHNICIAN.
- 9. BACKFILL IN PIPE TRENCHES SHALL BE PLACED IN LAYERS AND NOMINALLY COMPACTED.
- 10. WATER ENTERING THE EXCAVATION FROM SURFACE RUNOFF SHALL BE COLLECTED FROM THE EXCAVATION TO MAINTAIN A BOTTOM FREE FROM STANDING WATER.
- 11. WATER REMOVED FROM EXCAVATIONS SHALL BE MANAGED AND DISCHARGED INTO THE ONSITE LEACHATE MANAGEMENT SYSTEM AS DIRECTED BY THE OWNER OR CQA TECHNICIAN.
- 12. THE PREMISES SHALL BE KEPT FREE FROM ACCUMULATION OF WASTE MATERIALS OR RUBBISH CAUSED BY OPERATIONS. UPON COMPLETION OF THE CONSTRUCTION ACTIVITIES, ALL WASTE MATERIALS AND RUBBISH SHALL BE REMOVED, AS WELL AS ALL TOOLS, CONSTRUCTION EQUIPMENT, MACHINERY, AND SURPLUS MATERIALS.
- 13. PIPING SHALL NOT BE LAID IN WATER, AND NO PIPE SHALL BE LAID WHEN TRENCH OR WEATHER CONDITIONS ARE UNSUITABLE FOR SUCH WORK.
- 14. INSTALLED PIPES SHALL BE AIR PRESSURE TESTED AT A PRESSURE OF 5.0 PSIG. THE MAXIMUM ALLOWABLE PRESSURE LOSS SHALL BE 10 PERCENT OF THE STARTING PRESSURE OVER 1 HOUR. IN THE EVENT OF A FAILING TEST, THE PIPE AND FUSIONS SHALL BE INSPECTED FOR CRACKS, PINHOLES, OR PERFORATIONS VISUALLY OR BY UTILIZING A SOAP WATER MIXTURE.
- 15. PIPING SHALL BE BACKFILLED AND SECURED PRIOR TO AIR TESTING TO PREVENT DAMAGE TO ADJACENT PIPING AND EQUIPMENT IN THE EVENT OF A JOINT FAILURE. JOINTS SHALL BE BACKFILLED AFTER THE COMPLETION OF A PASSING AIR PRESSURE TEST AND APPROVAL BY THE CQA TECHNICIAN.
- 16. LENGTHS OF FUSED PIPING TO BE HANDLED SHALL NOT EXCEED 400 FEET.



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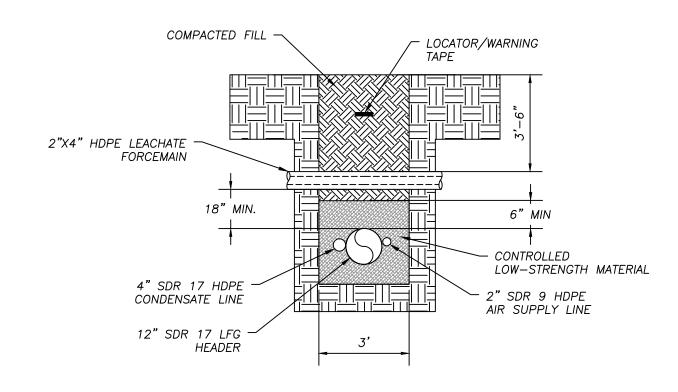
TAPE SHALL BE A METALLIC LOCATOR/WARNING TAPE IMPRINTED WITH "CAUTION GAS LINE"

BURIED BELOW," AS SUPPLIED BY TERRA TAPE, OR EQUIVALENT.

- 2. ALL HEADER AND LATERAL PIPING SHALL BE INSTALLED AT MIN. SLOPES IDENTIFIED ON PLANS UNLESS APPROVED IN ADVANCE BY ENGINEER.
- 3. THE NUMBER AND TYPES OF PIPES INSTALLED IN THE TRENCH MAY VARY. SEE SITE PLANS.

MINIMUM TRENCH WIDTH					
NOMINAL PIPE MINIMUM PARALLEL PIPE					
4" - 10"	24"	4"			
12" - 18"	PIPE O.D. + 14"	4''			

E HEADER/LATERAL TRENCH DETAIL
NOT TO SCALE

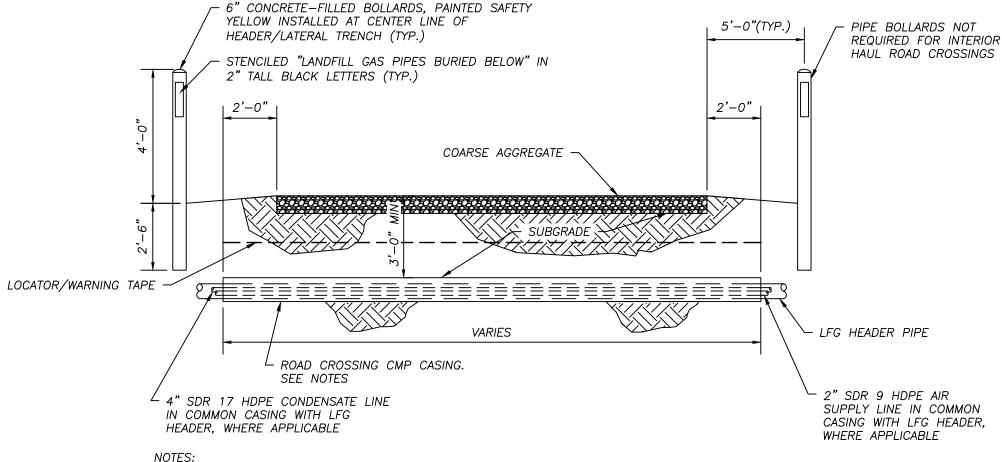


CLSM MIX DESIGN				
MATERIAL PERCENT BY WEIGH				
CEMENT	3			
BENTONITE	10			
FINE AGGREGATE/SILTY SANDY SOIL	77			
WATER	10			

#### <u>NOTE</u>

- 1. CONTROLLED LOW—STRENGTH MATERIAL (CLSM) SHALL CONSIST OF A MIXTURE OF FINE AGGREGATE/SILTY SANDY SOIL, CEMENT, BENTONITE, AND WATER WITH A DESIRED COMPRESSIVE STRENGTH OF 50—100 PSI. FINE AGGREGATE SHALL HAVE A MAXIMUM OF 30 PERCENT FINES. ALTERATIONS TO THE MIX DESIGN SHOWN SHALL BE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION.
- 2. THE MIXTURE SHALL BE ALLOWED TO HARDEN TO A POINT WHERE A PERSON MAY STAND ON THE MATERIAL PRIOR TO THE ADDITION OF ADDITIONAL FILL.
- 3. CLSM INSTALLATION SHALL BE A MINIMUM 6 INCHES ABOVE THE PIPE AND EXTEND 10 FEET IN EITHER DIRECTION OF THE HEADER/WATER LINE INTERSECTION.
- 4. TRENCHING NEAR EXISTING UTILITIES SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE OSHA, LOCAL, STATE, AND FEDERAL REGULATIONS.

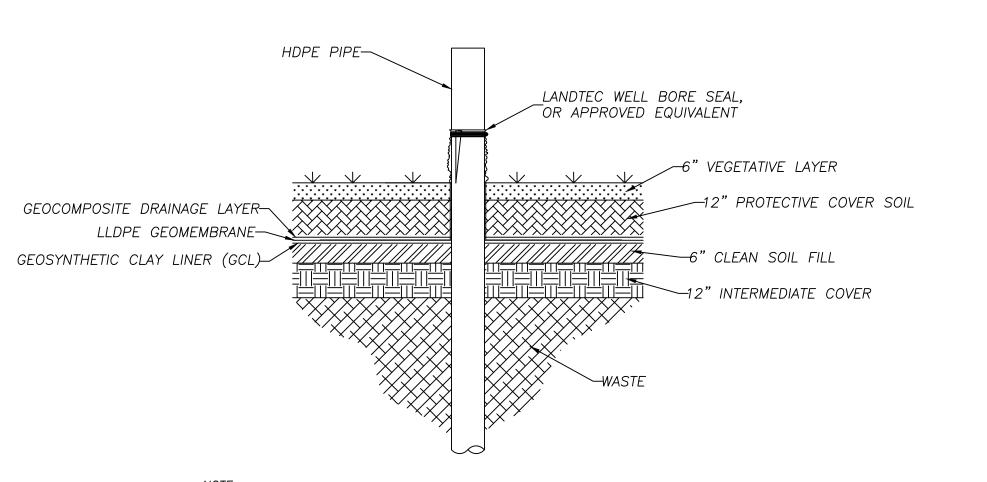




#### <u>NOTES</u>:

- 1. ROAD SHALL BE RESTORED TO MATCH ORIGINAL CONDITIONS.
- 2. CMP CASING SHALL BE A MINIMUM OF 5" GREATER THAN THE CUMULATIVE DIAMETER OF THE PIPES ENCASED.
- 3. SUBGRADE SHALL CONSIST OF 6" SOIL LIFTS COMPACTED TO A MINIMUM 95 PERCENT STANDARD PROCTOR A MINIMUM OF 18" DEEP AND SHALL EXTEND 2' BEYOND THE PROPOSED OUTSIDE EDGE OF GRAVEL.
- 4. HAUL ROAD CROSSINGS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THIS DETAIL BUT SHALL NOT INCLUDE THE CONCRETE BOLLARDS OR GRAVEL SURFACE.

G TYPICAL UNPAVED ROAD CROSSING DETAIL NOT TO SCALE



1. WELL BORE SEALS TO BE INSTALLED ON ALL HDPE PIPING PROTRUDING THROUGH THE FINAL COVER INCLUDING GAS EXTRACTION WELLS AND LATERAL RISER PIPES.

WELL BORE SEAL DETAIL
NOT TO SCALE

75% SUBMITTAL

SHEET TITLE

GCCS DETAILS 2

PROJECT TITLE

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CLOSURE OF INACTIVE NABORS LANDFILL

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A R K A N S A S
Department of Environmental Quality

SCS AQUATERRA
7311 W. 130th St, Ste. 100
Overland Park, Kansas 66213
PH. (913) 681-0030 FAX. (913) 681-0012

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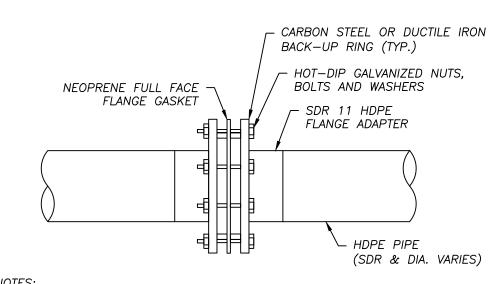
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7/9/15
DRAWING NO.

TEE FOR 12" DIAMETER RISER. FOR HEADERS 12" IN DIAMATER AND SMALLER, MATCH THE RISER DIAMETER TO THE HEADER.

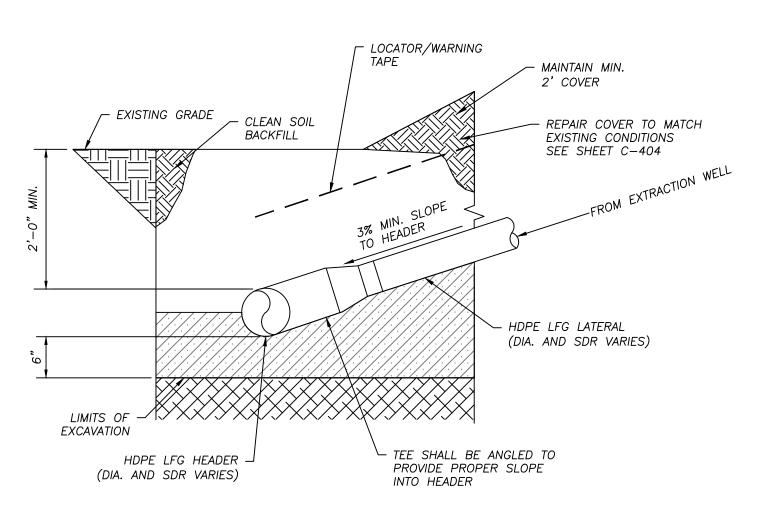
1. FOR HEADERS LARGER THAN 12" IN DIAMETER USE REDUCER



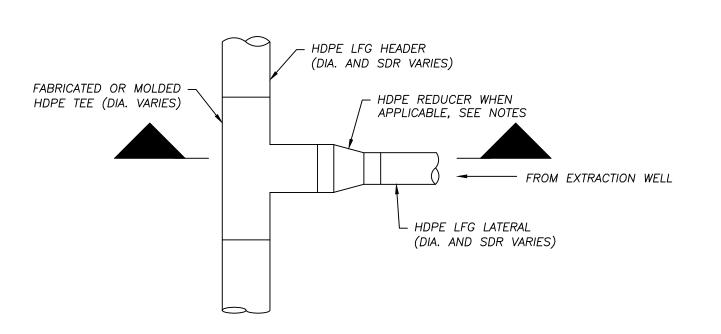


1. WHEN BUTT FUSION IS USED TO JOIN PIPES WITH DIFFERENT WALL THICKNESSES,
THEY MUST HAVE THE SAME OUTSIDE DIAMETER AND THE DIFFERENCE IN MINIMUM
WALL THICKNESS MUST NOT EXCEED 26 PERCENT.

FLANGE CONNECTION DETAIL NOT TO SCALE



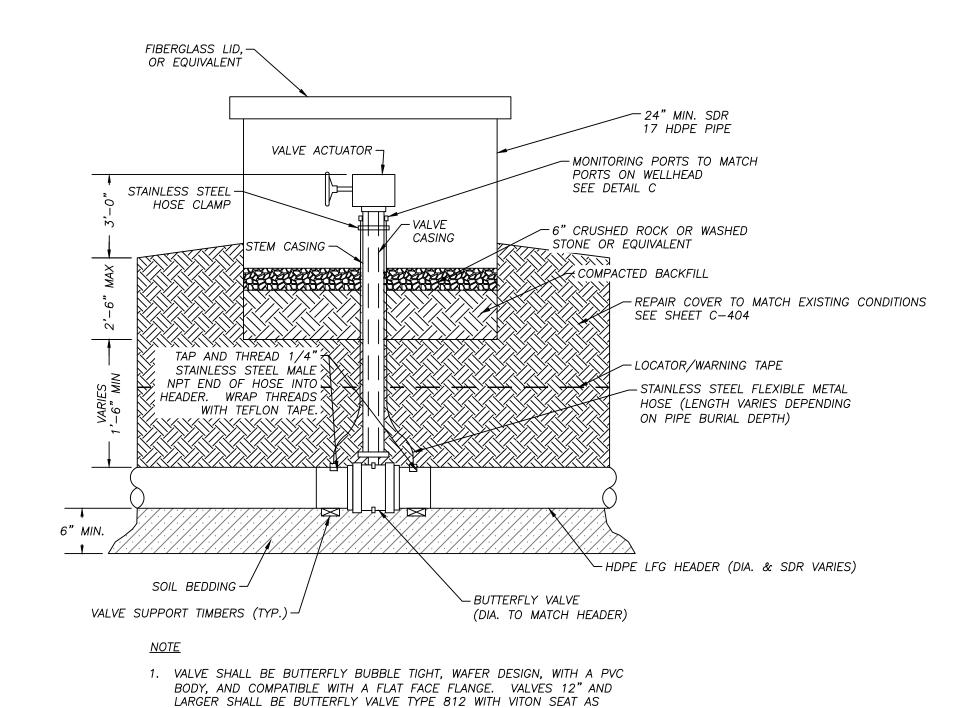
J LFG LATERAL TIE-IN WITH TEE DETAIL - SECTION VIEW NOT TO SCALE



#### <u>NOTE</u>

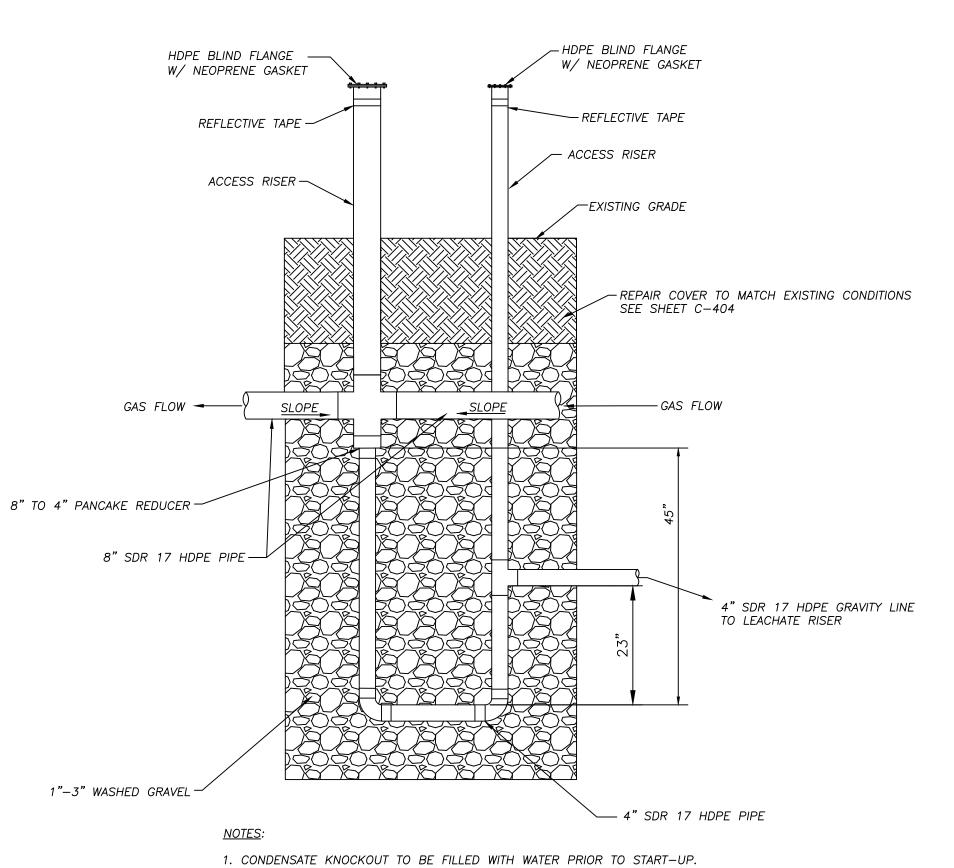
- 1. MOLDED HDPE TEES SHALL BE INSTALLED FOR ALL LATERAL TIE—INS SMALLER THAN 12" IN DIAMETER. TEES SHALL BE ANGLED TO MAINTAIN SLOPE INTO THE HEADER AS SHOWN IN SECTION VIEW.
- CONSECUTIVE SIZE REDUCERS SHALL BE USED TO TRANSITION FROM THE MOLDED TEE TO THE LFG LATERAL. FOR EXAMPLE,
  TO TRANSITION FROM A 12" TEE TO A 6" LATERAL, A 12"x10" REDUCER, 10"x8" REDUCER, AND 8"x6" REDUCER SHALL BE
  INSTALLED.
- 3. 1' MIN. OF CLEAN SOIL BEDDING SHALL BE PLACED BELOW EACH TEE. THE SOIL SHALL BE INSTALLED SO IT EXTENDS 3'
  MIN. IN EACH DIRECTION OF THE TEE. SOIL SHALL BE SLIGHTLY MOISTENED AND HAND—TAMPED PROVIDING SUPPORT TO
  ALL POINTS OF THE TEE. CLEAN, GRADED SOIL SHALL BE HAND—TAMPED ABOVE TEE (1' MIN.) BEING CAREFUL TO
  FILMINATE VOIDS.

J LFG LATERAL TIE-IN WITH TEE DETAIL - PLAN VIEW NOT TO SCALE



K TYPICAL SHUTOFF VALVE DETAIL
NOT TO SCALE

MANUFACTURED BY ABZ VALVE, OR EQUIVALENT.



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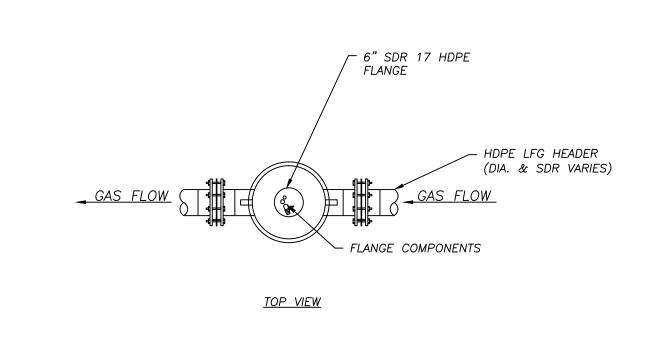
M TYPICAL CONDENSATE KNOCKOUT DETAIL

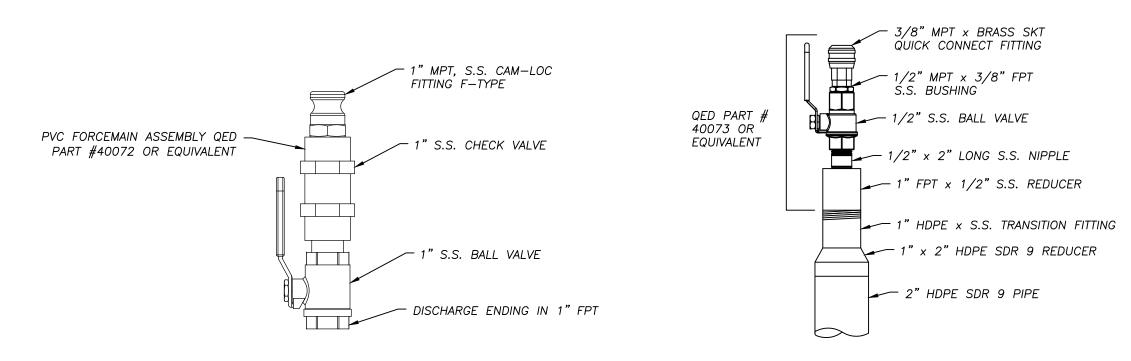
NOT TO SCALE

75% SUBMITTAL

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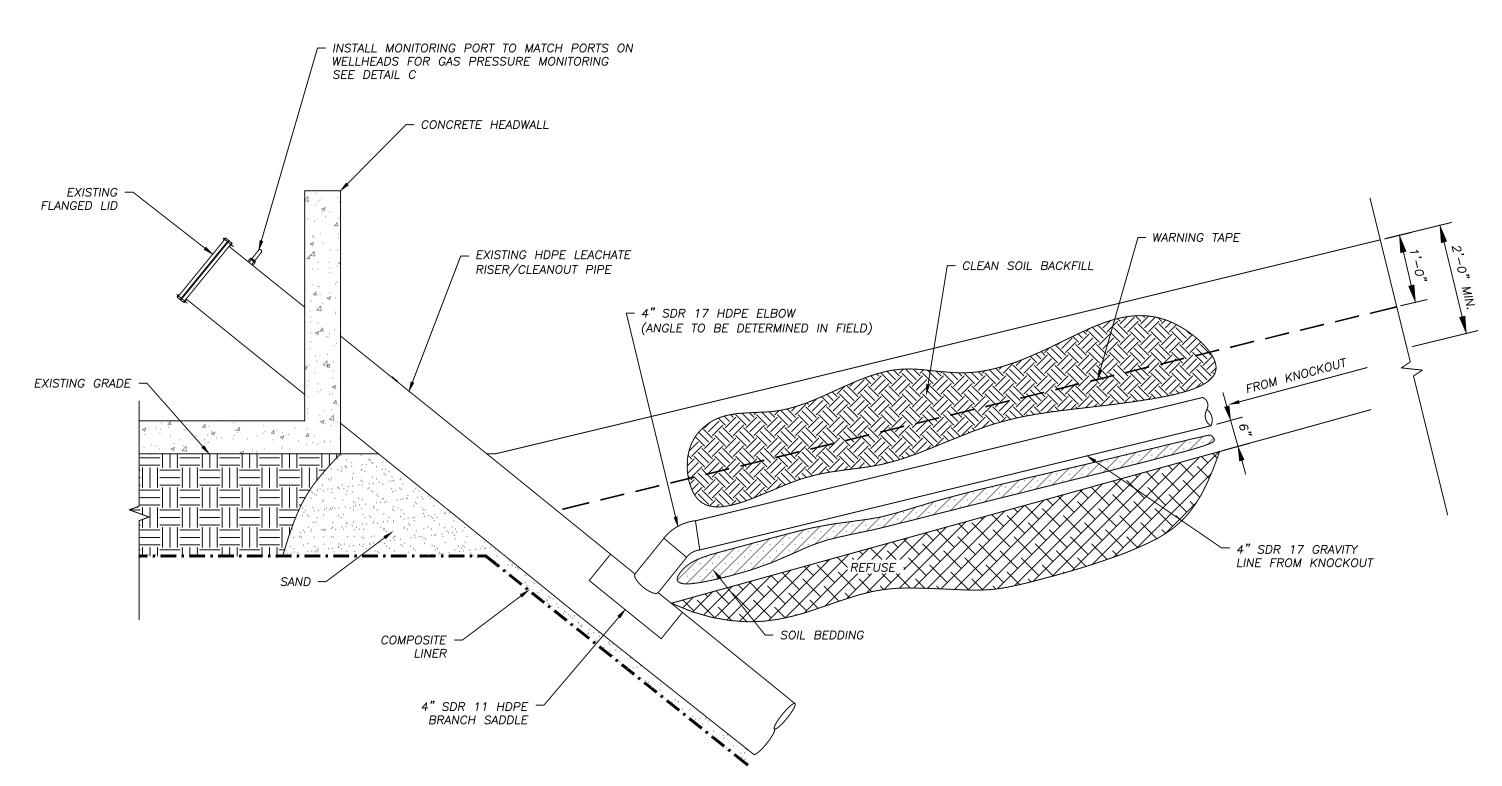
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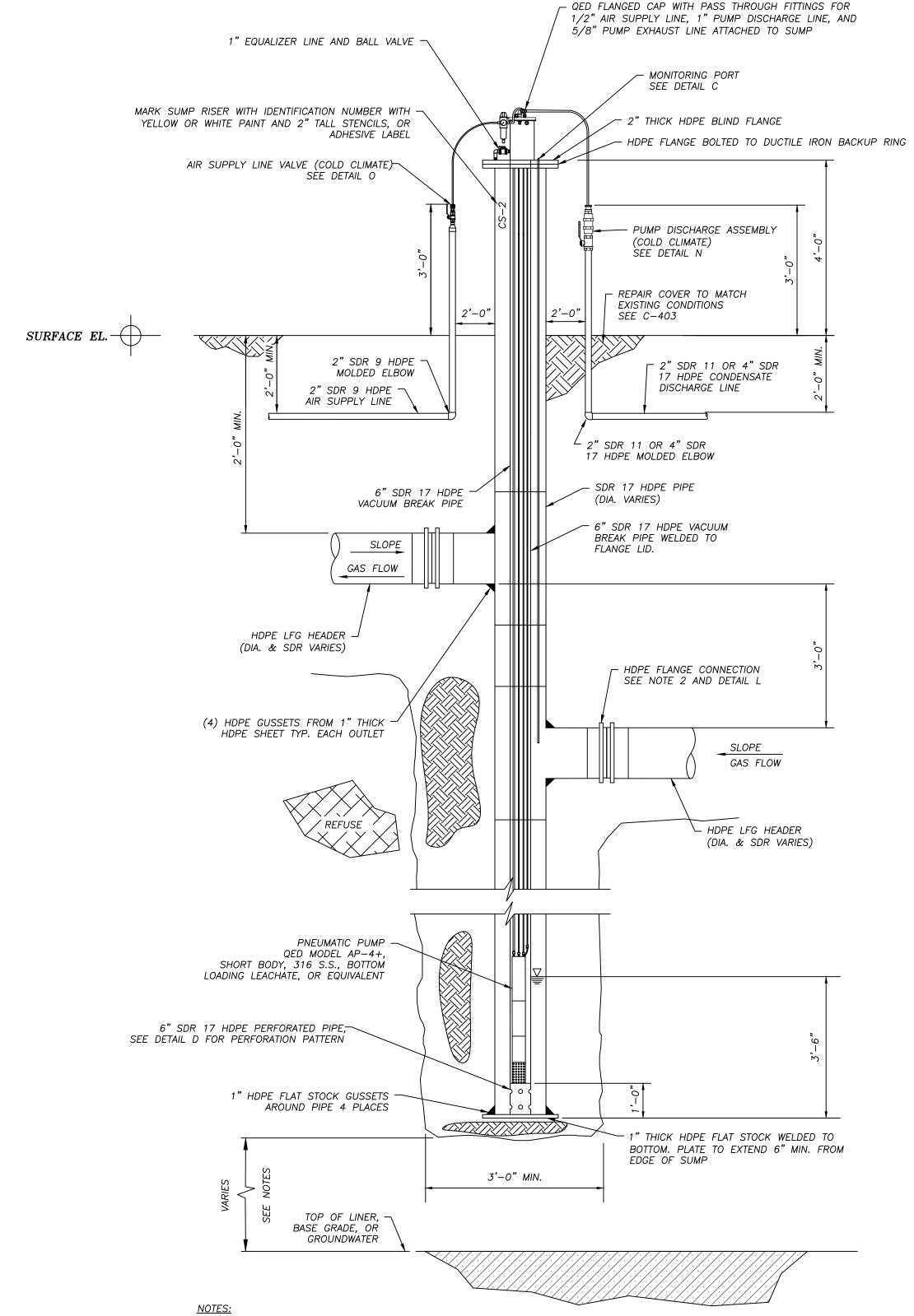




- 1. CAREFULLY EXCAVATE AROUND THE EXISTING LEACHATE/CLEANOUT RISER PIPING. MAKE TIE-IN A MINIMUM 10 FEET FROM EXISTING CONCRETE HEADWALL.
- 2. VERIFY LEACHATE COLLECTION RISER CONFIGURATION AND SIZE IN FIELD.

Q TYPICAL TIE-IN TO EXISTING LEACHATE/CLEANOUT RISER DETAIL





- 1. 6" DIA. SDR 17 HDPE VACUUM BREAK PIPE EXTRUSION WELDED TO FLANGE LID. 1'-0" OF PIPE TO PROTRUDE THROUGH TOP OF FLANGE. EXTRUSION WELD PIPE TO BOTH SIDES OF BLIND FLANGE. CENTER OF PIPE LOCATED 2" OFF CENTER OF BLIND FLANGE.
- 2. THOROUGHLY COAT ENTIRE SURFACE OF BOLTS, WASHERS, NUTS AND BACKUP RINGS WITH POLYCOAT RUBBERIZED PRIMER, OR EQUIVALENT, AFTER TIGHTENING BOLTS. WRAP FLANGE IN PLASTIC WRAP PRIOR TO BACKFILLING.
- 3. VERIFY CONDENSATE SUMP CONFIGURATION IN FIELD. ADJUSTMENTS SHALL BE APPROVED BY ENGINEER PRIOR TO INSTALLATION.
- 4. ENGINEER SHALL APPROVE DEPTH BASED ON GROUND SURFACE ELEVATION PRIOR TO INSTALLATION.
- 5. UNDER NO CIRCUMSTANCE SHALL THE SUMP BE INSTALLED LESS THAN 10 FEET FROM THE BASE GRADE ELEVATION.

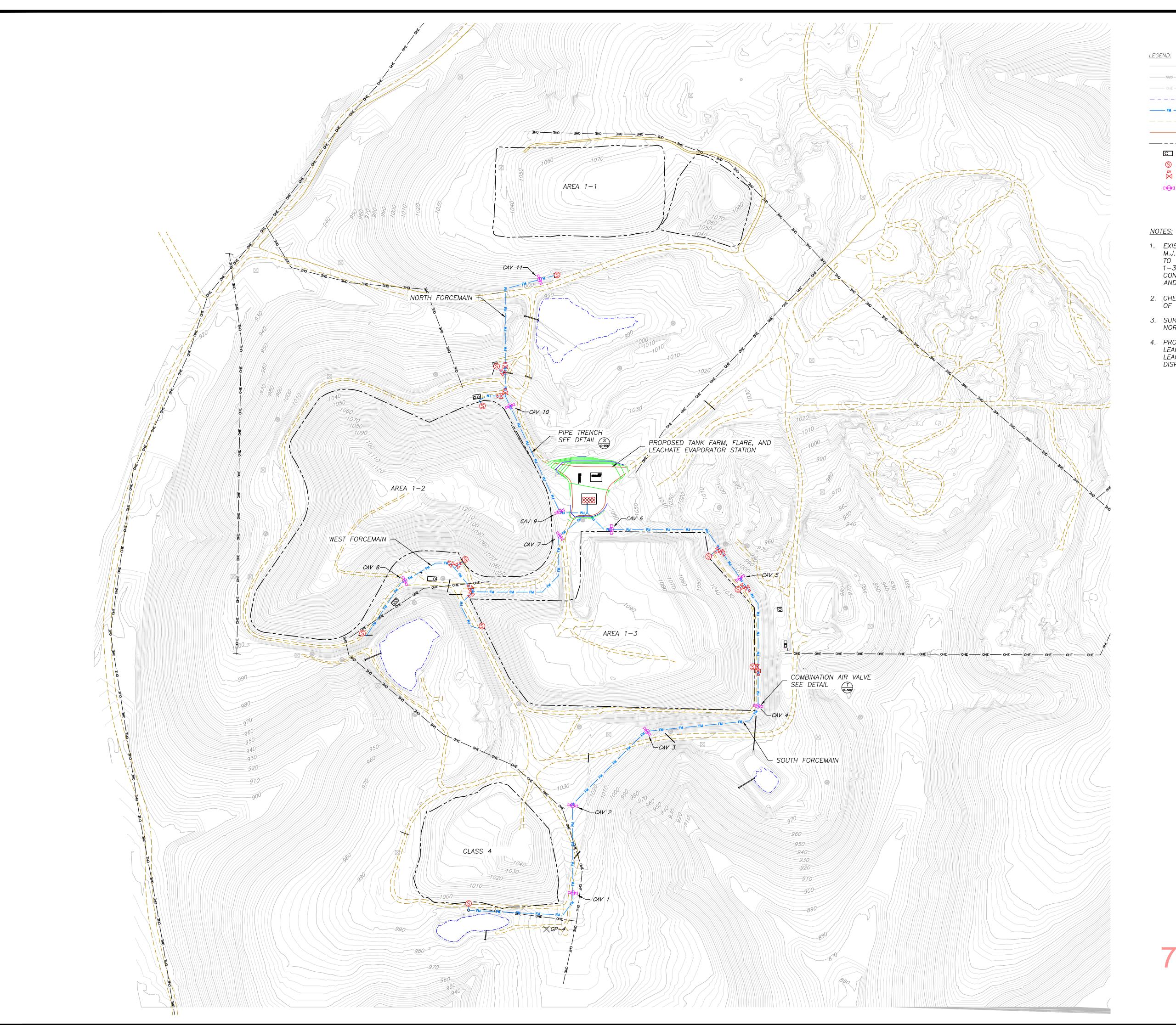


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7311 W. 130th St, Ste. 100 Overland Park. Kansas 66213					
PH. (913) 681-0030 FAX. (913) 681-0012					
>d wind	ARKANSAS	PROJECT TITLE			
14218.01 CMM: B1: ZDB	ä	<b>CLOSURE OF INACTIVE NABORS LANDFILL</b>	1 7/2015	75% SUBMITTAL	FEC
DSN. BT: ZDB CIN. BT: FEC FROG. MGR FEC			0 5/2015	50% SUBMITTAL	FEC

CADD FILE:

7/9/15 DRAWING NO.



EXISTING 5' MINOR CONTOUR

EXISTING 25' MAJOR CONTOUR

EXISTING OVERHEAD ELECTRIC

EXISTING LEACHATE STORAGE TANK AND SECONDARY CONTAINMENT

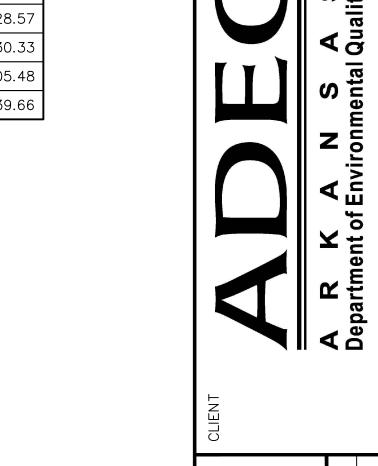
S PROPOSED LEACHATE SUMP/COLLECTION POINT

PROPOSED CHECK VALVE

PROPOSED COMBINATION AIR VALVE

- EXISTING TOPOGRAPHY BASED ON AERIAL SURVEY PERFORMED BY M.J. HARDEN ASSOCIATES, INC ON AUGUST 17, 2010. UPDATES TO THE TOPOGRAPHY AND SITE FEATURES FOR AREA 1-2, AREA 1-3, AND SOUTH CLASS 4 DISPOSAL UNITS SURVEYED BY CONSOLIDATED LAND SERVICES, INC BETWEEN DECEMBER 2014 AND JANUARY 2015.
- CHECK VALVES ARE ORIENTATED WITH "CV" TO THE INFLOW SIDE OF THE VALVE.
- 3. SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.
- 4. PROPOSED LEACHATE COLLECTION SUMPS BASED ON EXISTING LEACHATE COLLECTION POINTS. CONNECTION TO EXISTING LEACHATE COLLECTION SUMPS WILL DIFFER BASED ON THE DISPOSAL UNIT.

Air Valve Locations						
Point #	Description	Northing	Easting			
101	CAV 1	772973.82	1180790.33			
102	CAV 2	773370.56	1180789.33			
103	CAV 3	773706.57	1181126.57			
104	CAV 4	773821.52	1181630.18			
105	CAV 5	774401.71	1181548.36			
106	CAV 6	774622.36	1180964.77			
107	CAV 7	774592.93	1180731.36			
108	CAV 8	774389.01	1180028.57			
109	CAV 9	774703.40	1180730.33			
110	CAV 10	775182.20	1180505.48			
111	CAV 11	775756.00	1180639.66			



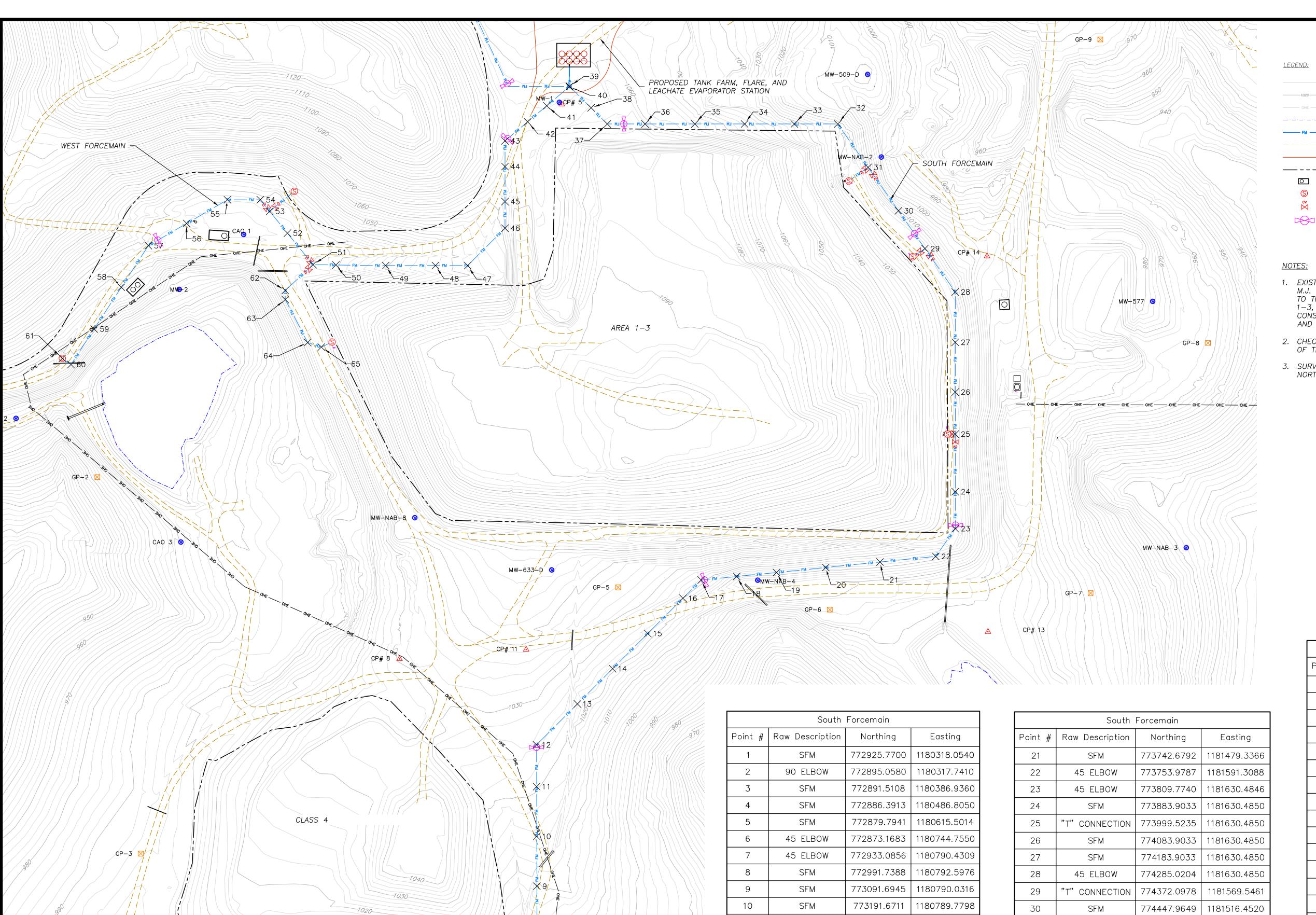
LAYOU

SCS AQUATERRA
7311 W. 130th St, Ste. 100
Overland Park, Kansas 66213
PH. (913) 681-0030 FAX. (913) 681-0012

J. NO.
27214218.01
DWN. BY: DRS Q/A RVW BY

ADD FILE: –500 – Leachate Forcemain Layout.dw

7/9/20



SFM

45 ELBOW

SFM

SFM

SFM

SFM

45 ELBOW

SFM

SFM

15

19

20

773291.6477 | 1180789.5281

773375.4843 | 1180789.2990

773457.6270 | 1180871.4200

773528.3485 | 1180942.1200

773599.0699 | 1181012.8200

773669.7914 | 1181083.5200

773721.6967 | 1181271.4099

773731.6700 1181370.2410

1181120.1500

1181191.4583

773706.4325

773713.6285

South Forcemain					
Point #	Raw Description	Northing	Easting		
21	SFM	773742.6792	1181479.3366		
22	45 ELBOW	773753.9787	1181591.3088		
23	45 ELBOW	773809.7740	1181630.4846		
24	SFM	773883.9033	1181630.4850		
25	"T" CONNECTION	773999.5235	1181630.4850		
26	SFM	774083.9033	1181630.4850		
27	SFM	774183.9033	1181630.4850		
28	45 ELBOW	774285.0204	1181630.4850		
29	"T" CONNECTION	774372.0978	1181569.5461		
30	SFM	774447.9649	1181516.4520		
31	"T" CONNECTION	774535.1189	1181455.4591		
32	45 ELBOW	774622.3611	1181394.4050		
33	SFM	774622.3606	1181307.2650		
34	SFM	774622.3600	1181207.2650		
35	SFM	774622.3595	1181107.2650		
36	SFM	774622.3589	1181007.2650		
37	45 ELBOW	774622.3585	1180931.4063		
38	SFM	774654.8634	1180898.9013		
39	45 ELBOW	774697.9143	1180855.8504		

EXISTING	5'	MINOR	CONTOUR

EXISTING 25' MAJOR CONTOUR

EXISTING OVERHEAD ELECTRIC EXISTING BODY OF WATER

PROPOSED FORCEMAIN EXISTING ROADWAY

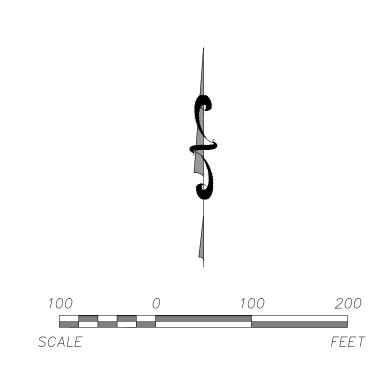
PROPOSED ROADWAY

—— -- DISPOSAL BOUNDARY (APPROXIMATE) EXISTING LEACHATE STORAGE TANK AND SECONDARY CONTAINMENT

PROPOSED LEACHATE SUMP/COLLECTION POINT CHECK VALVE

COMBINATION AIR VALVE

- 1. EXISTING TOPOGRAPHY BASED ON AERIAL SURVEY PERFORMED BY M.J. HARDEN ASSOCIATES, INC ON AUGUST 17, 2010. UPDATES TO THE TOPOGRAPHY AND SITE FEATURES FOR AREA 1-2, AREA 1-3, AND SOUTH CLASS 4 DISPOSAL UNITS SURVEYED BY CONSOLIDATED LAND SERVICES, INC BETWEEN DECEMBER 2014 AND JANUARY 2015.
- 2. CHECK VALVES ARE ORIENTATED WITH "CV" TO THE INFLOW SIDE OF THE VALVE.
- 3. SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.



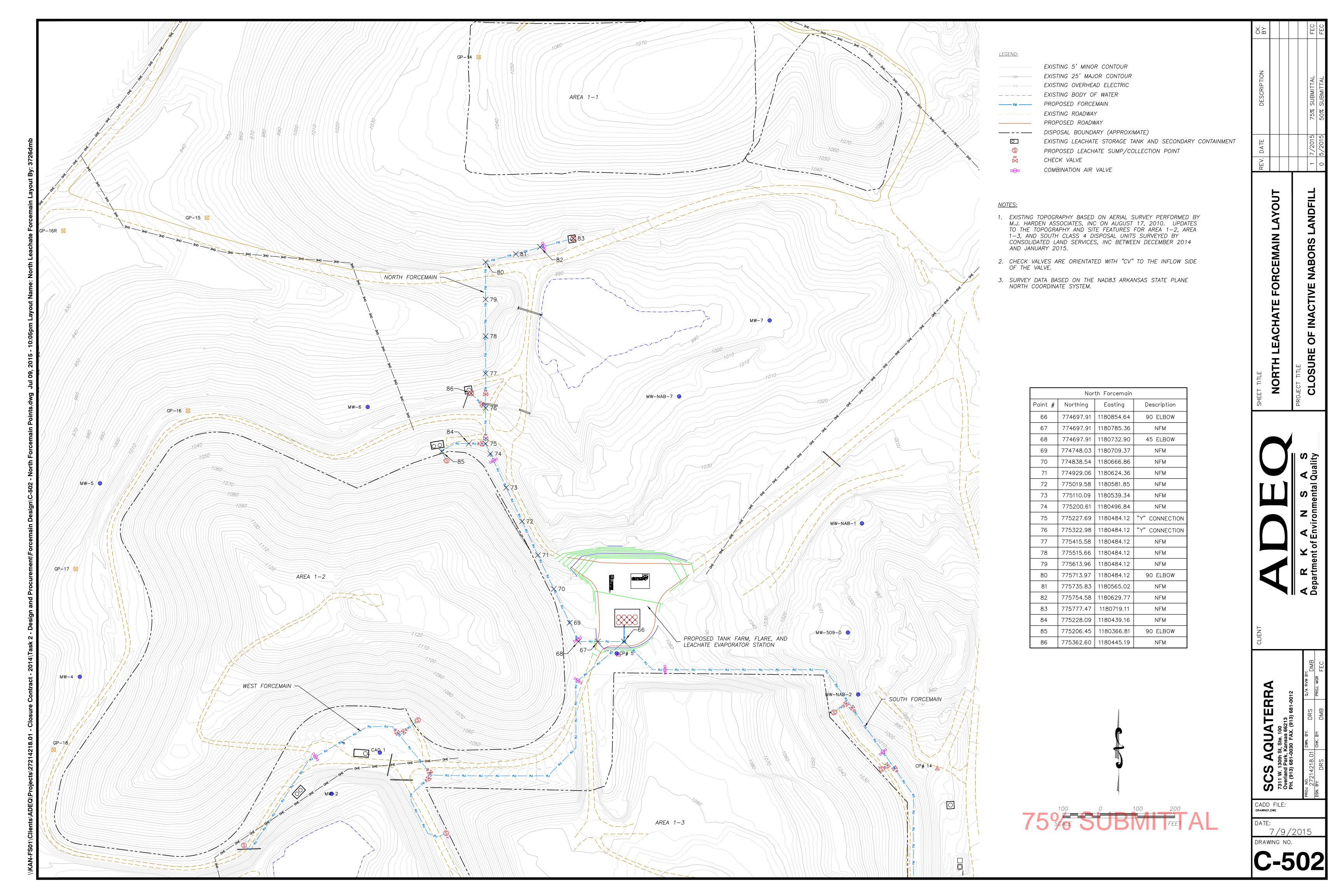
West Forcemain					
Point #	Raw Description	Northing	Easting		
40	45 ELBOW	774697.0948	1180855.3164		
41	WFM	774658.9040	1180809.8661		
42	WFM	774626.6391	1180771.4681		
43	45 ELBOW	774588.4054	1180725.9668		
44	WFM	774536.8363	1180725.9668		
45	WFM	774466.4964	1180725.9668		
46	45 ELBOW	774413.0311	1180725.9670		
47	45 ELBOW	774338.0311	1180650.9670		
48	WFM	774338.0311	1180586.2720		
49	WFM	774338.0311	1180486.2720		
50	WFM	774339.1276	1180386.2800		
51	"Y" CONNECTION	774339.8295	1180340.3000		
52	WFM	774403.1243	1180288.8850		
53	"T" CONNECTION	774446.9200	1180253.3090		
54	45 ELBOW	774470.0959	1180234.4830		
55	45 ELBOW	774470.0959	1180168.3420		
56	WFM	774422.5784	1180086.4380		
57	45 ELBOW	774377.9528	1180009.5180		
58	WFM	774294.4229	1179955.1050		
59	WFM	774210.5286	1179900.6830		
60	90 ELBOW	774139.3852	1179854.1100		
61	WFM	774151.7233	1179836.6820		
62	WFM	774288.0311	1180284.4830		
63	WFM	774269.7958	1180284.4830		
64	WFM	774183.4984	1180329.5090		
65	WFM	774174.2709	1180356.8620		

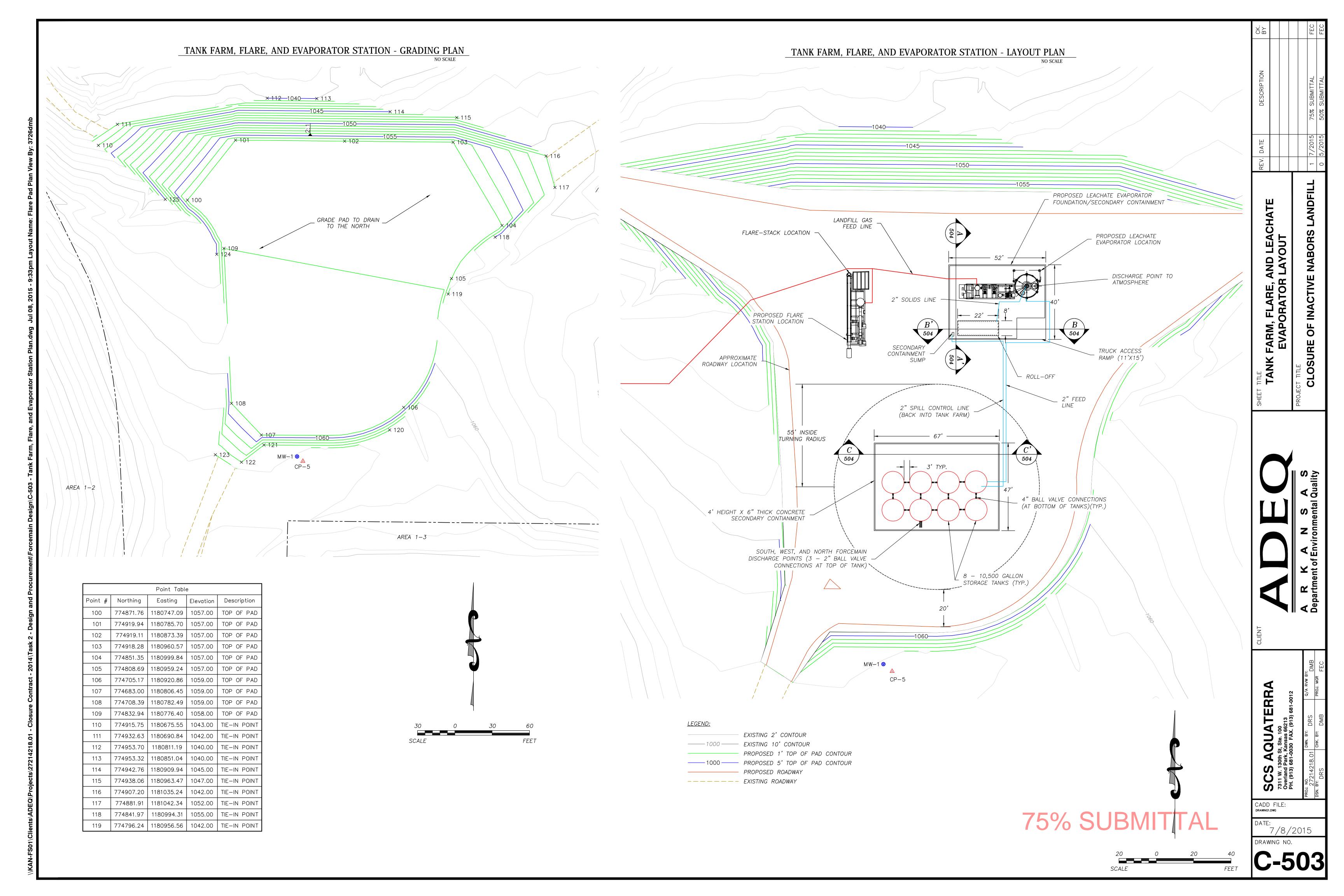
DESCRIPTION			75% SUBMITTAL	50% SUBMITTAL	
REV. DATE			7/2015	0 5/2015	
REV.			1	0	
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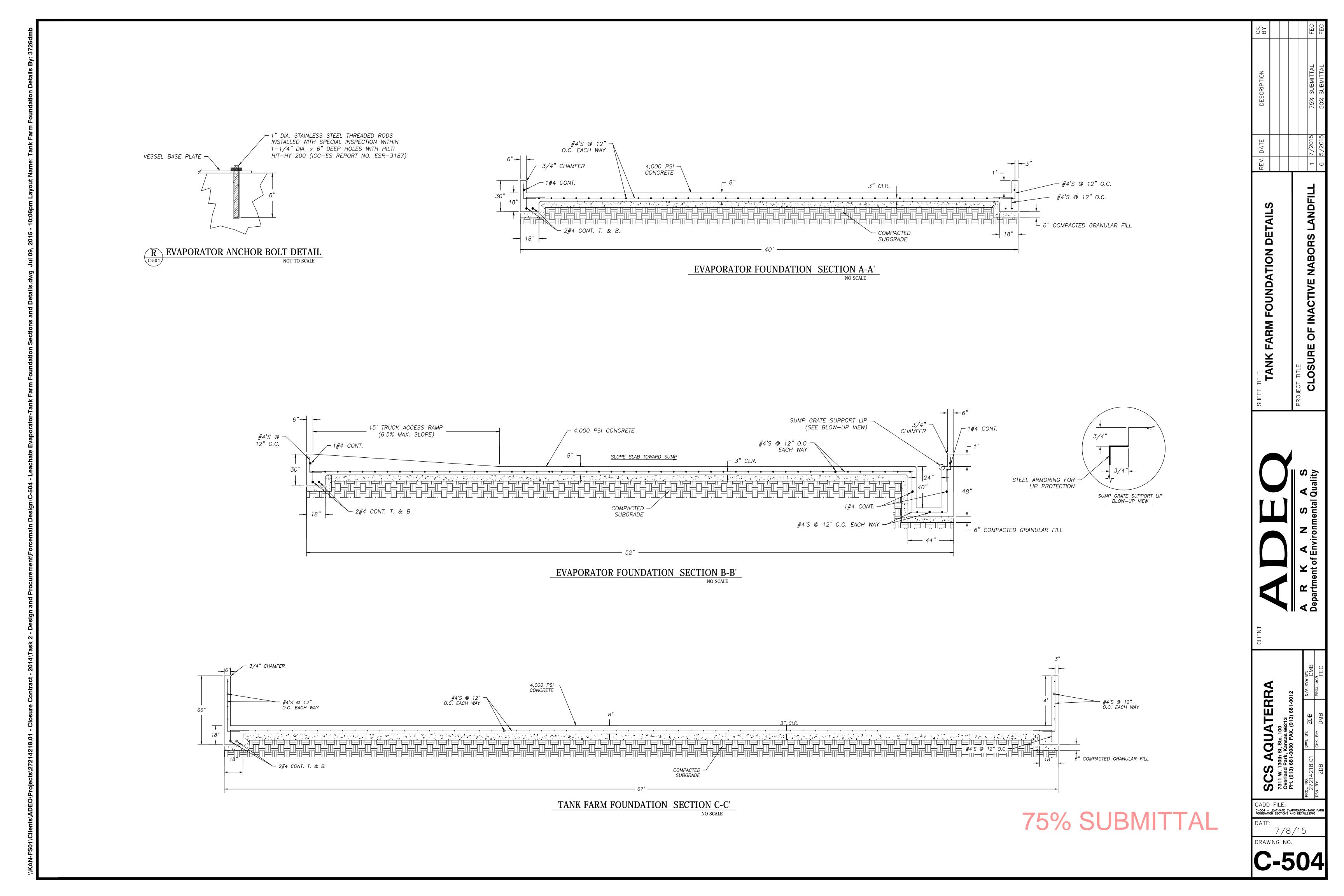
AQUATERR

CADD FILE: drawing1.dwg

DATE: 7/9/2015







#### **FOUNDATION:**

- 1. SOIL DESIGN PRESSURE: 1,500 P.S.F.
- 2. FOOTINGS SHALL EXTEND AT LEAST 36" BELOW FINISHED GRADE TO PROVIDE ADEQUATE FROST PROTECTION.
- 3. SOIL ENGINEER SHALL VERIFY THAT CONSTRUCTION AT THE SITE IS IN ACCORDANCE WITH THE RECOMMENDATIONS AND CONCLUSIONS OF HIS REPORT.
- 4. FINISHED EXCAVATIONS FOR FOUNDATION SHALL BE NEAT AND TRUE TO LINE WITH ALL LOOSE MATERIALS AND STANDING WATER REMOVED FROM EXCAVATION.
- 5. BEFORE ANY CONCRETE IS PLACED, EXCAVATIONS SHALL BE CHECKED AND APPROVED BY A QUALIFIED SOILS ENGINEER TO INSURE COMPLIANCE WITH THE REQUIREMENTS.
- 6. ALL FILL MATERIAL IS TO BE APPROVED BY THE SOILS ENGINEER AND IS TO BE COMPACTED TO 95% OF MAXIMUM DENSITY INSPECTION IS REQUIRED DURING FILL AND COMPACTION.
- 7. NOTIFY THE STRUCTURAL ENGINEER OF ANY UNUSUAL SOIL CONDITIONS THAT ARE IN VARIANCE WITH THE DATA HEREIN
- 8. TEMPORARY EXCAVATION SLOPES IN DRY, LOOSE DEPOSITS SHOULD BE MAINTAINED NOT STEEPER THAN 1.5 HORIZONTAL TO 1.0 VERTICAL (1.5 : 1). IN WET OF LOOSE SOILS, TEMPORARY EXCAVATION SLOPES SHOULD BE FLATTENED AS REQUIRED IN ACCORDANCE WITH OSHA REGULATIONS 29 CFR PART 1926.
- 9. FOLLOWING EXCAVATION TO AT LEAST 12 INCHES BELOW BOTTOM OF FOOTING, THE SUBGRADE SHOULD THEN BE COMPACTED IN ORDER TO DENSIFY ANY NATURALLY OCCURRING LOOSE ZONES OR THOSE WHICH DEVELOP DURING THE
- 10. FOLLOWING COMPACTION OPERATIONS, BACKFILL AS REQUIRED, TO THE BOTTOM OF THE PROPOSED EQUIPMENT FOOTINGS AND MAT FOUNDATIONS WITH SELECT GRANULAR FILL MATERIAL. THIS MATERIAL SHOULD BE PLACED IN 12-INCH MAXIMUM LOOSE LIFTS AND BE COMPACTED TO A MINIMUM OF 95 PERCENT OF THE MATERIALS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557.
- 11. EQUIPMENT FOUNDATIONS SHOULD BE DESIGNED AND SIZED TO ADEQUATELY DAMPEN DYNAMIC FORCES AND VIBRATIONS CAUSED BY THE OPERATION OF EQUIPMENT SUPPORTED ON THE FOUNDATION. THE NATURAL FREQUENCY OF VIBRATION SHOULD DIFFER FROM THE OPERATING FREQUENCY BY AT LEAST 50 PERCENT TO AVOID RESONANCE.

#### REINFORCED CONCRETE:

- 1. CEMENT FOR CONCRETE OR GROUT SHALL CONFORM TO A.S.T.M. C-150, TYPE II.
- 2. AGGREGATES SHALL CONFORM TO A.S.T.M. C-33 FOR NORMAL WEIGHT CONCRETE AND A.S.T.M. C-330 FOR LIGHTWEIGHT CONCRETE.
- 3. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH A.S.T.M. C-94.
- 4. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST A.C.I. CODE (A.C.I. 318) AND DETAILING MANUAL (A.C.I. 315) UNLESS OTHERWISE DETAILED OR NOTED IN DRAWINGS.
- 5. CONCRETE SHALL HAVE A MINIMUM ULTIMATE STRENGTH AT 28 DAYS f'C OF 4,000 PSI, WITH A WATER CEMENT RATIO NOT EXCEEDING 0.50. SUBMIT CONCRETE MIX DESIGNS TO THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL. CONCRETE DESIGN IS BASED ON f'C=2,500 PSI AND, THEREFORE, SPECIAL INSPECTION OF CONCRETE IS NOT REQUIRED.
- 6. ADMIXTURES MAY BE USED WITH APPROVAL OF THE ENGINEER. ADMIXTURES USED TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT.
- 7. PROJECTING CORNERS OF SLABS, SHALL BE FORMED WITH A 3/4" CHAMFER UNLESS OTHERWISE NOTED.
- 8. RIGID, REINFORCED CONCRETE SPREAD FOOTINGS AND MAT FOUNDATIONS SUPPORTED UPON SELECT GRANULAR FILL CAN BE PROPORTIONED USING AN ALLOWABLE NET BEARING PRESSURE OF 1,500 POUNDS PER SQUARE FOOT (PSF). THIS BEARING PRESSURE ASSUMES THAT SPREAD FOOTINGS WILL HAVE A MINIMUM WIDTH OF 1.5 FEET. FOOTINGS DESIGNED USING THE RECOMMENDATIONS HEREIN SHOULD NOT EXPERIENCE TOTAL SETTLEMENT GREATER THAN 0.75 INCHES. DIFFERENTIAL SETTLEMENT ACROSS THE MAT FOUNDATIONS SHOULD NOT EXCEED 0.50 INCHES.
- 9. CONCRETE FORM TOLERANCES SHALL BE WITHIN THE STANDARDS SET BY THE AMERICAN CONCRETE INSTITUTE.
- 10. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER INSERTS SHALL BE WELL SECURED IN PLACE IN THE FORMS PRIOR TO PLACING OF CONCRETE. TWO—WAY MATS OF STEEL MUST BE WIRED TOGETHER BOTH WAYS AT ALTERNATE INTERSECTIONS.
- 11. LOCATION OF CONSTRUCTION JOINTS NOT SPECIFICALLY INDICATED ON DRAWINGS SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACING REINFORCING STEEL.
- 12. CONCRETE SLABS VARIATION FROM LEVEL TO BE 1/16" IN TEN FEET MAXIMUM, UNLESS OTHERWISE NOTED ON DRAWINGS.
- 13. PIPES MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES, BUT NOT BE EMBEDDED THEREIN.
- 14. DRYPACK SHALL CONSIST OF ONE PART PORTLAND CEMENT, 4 PARTS SAND BASED ON DRY LOOSE VOLUMES AND NOT LESS THAN 1/4 PART NOR MORE THAN 1/2 PART LIME PUTTY OR HYDRATED LIME. DRYPACK SHALL OBTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS. SUBMIT MIX DESIGN TO THE STRUCTURAL ENGINEER FOR REVIEW.
- 15. MINIMUM EMBEDMENT OF ANCHOR BOLTS (A.B.) UNLESS OTHERWISE NOTED ON THE PLANS, SHALL BE 7" IN FOOTINGS.

  ALL BOLTS SHALL HAVE A STANDARD BOLT HEAD OR A 1-1/4" 90° BEND AT EMBEDDED END. ANCHOR BOLTS SHALL BE SPACED A MINIMUM OF 12 DIAMETERS. IN LIEU OF BOLTS OR DOWELS IN CONCRETE, APPROVED CAST-IN-PLACE THREADED INSERTS MAY BE USED.
- 16. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ INSTALLED WITH SPECIAL INSPECTION IN ACCORDANCE WITH ICC/ES REPORT No. 1917, OR APPROVED EQUAL.
- 17. EPOXY ANCHORS SHALL BE THREADED RODS INSTALLED WITH SPECIAL INSPECTION WITH HILTI HY-200 EPOXY ADHESIVE IN ACCORDANCE W/ ES REPORT No. ESR-3187, OR APPROVED EQUAL.

#### **GENERAL:**

- 1. THE CONTRACTOR SHALL TAKE NO ADVANTAGE OF ANY ERROR OR OMISSION IN THE PLANS, ESTIMATED QUANTITIES OR SPECIFICATIONS. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS ON THE JOB SITE PRIOR TO COMMENCING WITH THE WORK. SPECIAL CARE SHALL BE GIVEN TO SITE AND BUILDING LAYOUT THEREON. IN THE EVENT THE CONTRACTOR DISCOVERS AN ERROR, OMISSION, OR POSSIBLE DISCREPANCY BETWEEN FIELD CONDITIONS AND THE DRAWINGS, THEY SHALL IMMEDIATELY NOTIFY THE ENGINEER PRIOR TO PROCEEDING WITH THE WORK.
- 2. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.
- 3. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE 2012 EDITION AND THE LOCAL BUILDING OFFICIAL.
- 4. THE DESIGN, ADEQUACY AND SAFETY OF ERECTION, BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND HAS NOT BEEN CONSIDERED BY THE STRUCTURAL ENGINEER.
- 5. NO PIPES, DUCTS, SLEEVES, CHASES, ETC. SHALL BE PLACED IN SLABS, BEAMS, OR WALLS UNLESS SPECIFICALLY SHOWN OR NOTED. NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES DUCTS, ETC., UNLESS OTHERWISE NOTED. CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FOR INSTALLATION OF ANY ADDITIONAL PIPES, DUCTS ETC., REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS.
- 6. ALL DETAIL CALLOUTS AS SHOWN ON THE DRAWINGS, SECTIONS AND ELEVATIONS SHALL APPLY TO ALL SIMILAR CONDITIONS WHETHER REFERENCED OR NOT. TYPICAL DETAILS AND NOTES SHALL APPLY UNLESS SHOWN OTHERWISE IN DRAWINGS
- 7. DESIGN LOADS:

  WIND LOADS:

  BASIC WIND SPEED 105 MPH

  WIND IMPORTANCE I = 1.0

  WIND EXPOSURE C

  EARTHQUAKE:

  SEISMIC IMPORTANCE FACTOR I = 1.0

  Ss = 0.507 g

  S1 = 0.164 g

  SITE CLASS:

 $SDs = 0.231 \ q$ 

SEISMIC CATEGORY C

SD = 0.077 q

- 8. NO CHANGES SHALL BE MADE TO THESE DRAWINGS WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.
- 9. WORK THESE DRAWINGS WITH CIVIL, MECHANICAL, PROCESS, GENERAL AND ELECTRICAL DRAWINGS.
- 10. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS SUCH STANDARDS SHALL BE THE LATEST EDITION, AND/OR ADDENDUM.
- 11. AS A CONVENIENCE TO THE CONTRACTOR, THE ENGINEER SHALL REVIEW SHOP DRAWINGS AS TO THEIR GENERAL CONFORMANCE TO THE DESIGN CONCEPT. THE CONTRACTOR SHALL BE RESPONSIBLE NONETHELESS FOR COMPLIANCE AND DIMENSIONS. SHOP DRAWINGS ARE REQUIRED FOR THE FOLLOWING:

REINFORCING STEEL HIGH STRENGTH BOLTING STRUCTURAL FIELD WELDING EPOXY ANCHORS

- 12. FOR EXCAVATION IN NATIVE SOIL, SHORING SHALL BE PROVIDED TO SATISFY STATE OF ARKANSAS SAFETY REQUIREMENTS.
- 13. ALL FILLS MUST BE COMPACTED TO AT LEAST 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR PROCEDURES OUTLINED IN ASTM D698.
- 14. BACKFILL MATERIAL AROUND THE FOUNDATION ELEMENTS SHOULD CONSIST OF SUITABLE ON—SITE MATERIAL OR CLEAN GRANULAR FILL FROM A BORROW SOURCE. THIS MATERIAL SHOULD BE PLACED IN 12—INCH MAXIMUM LOOSE LIFTS AND BE COMPACTED TO A MINIMUM OF 95 PERCENT OF THE MATERIALS MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D—1557.

#### REINFORCING STEEL:

- 1. ALL REINFORCING STEEL UNLESS OTHERWISE NOTED IN DRAWINGS SHALL CONFORM TO ASTM A-615, GRADE 60.
- 2. REINFORCING DETAILING, BENDING AND PLACING SHALL BE IN ACCORDANCE WITH CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST FOITION.
- 3. REINFORCING SHALL BE SPLICED ONLY AS SHOWN OR NOTED. SPLICES AT OTHER LOCATIONS MAY BE ALLOWED ONLY IF APPROVED BY THE STRUCTURAL ENGINEER.
- 4. PLACING OF REINFORCEMENT SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE'S BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, LATEST EDITION.
- 5. DIMENSIONS FROM FACE OF CONCRETE TO STEEL (CLR.) ARE TO FACE OF BARS.
- 6. THE TRANSVERSE REINFORCING STEEL SHALL TERMÎNATÉ ONE AND ONE—HALF INCHES FROM THE CONCRETE SURFACE.
- 7. BARS NOTED "CONT" AND TYPICAL WALL REINFORCING SHALL HAVE A MINIMUM SPLICE EQUAL TO THE STANDARD LAP SPLICES UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
- 8. REINFORCING BARS AND ACCESSORIES SHALL NOT BE IN CONTACT WITH ANY PIPE, PIPE FLANGE OR METAL PARTS EMBEDDED IN CONCRETE. A MINIMUM CLEARANCE OF 2" SHALL
- BE MAINTAINED BETWEEN REINFORCING STEEL AND ALL EMBEDDED METAL PARTS.

  9. PROVIDE THE MINIMUM PROTECTIVE COVERING OF CONCRETE UNLESS OTHERWISE NOTED:

BELOW GRADE OR EXPOSED TO WEATHER:
UNIFORMED, CAST IN PLACE
FORMED, CAST IN PLACE
2" CLEAR

LAP SCHEDULE—CLASS "B"					
BAR SIZE	TOP BARS	OTHER BARS			
#3	<i>30"</i>	23"			
#4	41"	31"			
<b>#</b> 5	51"	39"			
#6	61"	47"			
#7	89"	68"			
#8	101"	78"			

BEND SCHEDU	ILE
BAR SIZES	D
#3 THRU #8	6d

D = DIAMETER OF REINF. BAR

#### NOTES:

- 1. CONCRETE CLASS: 3,500 PSI NORMAL WEIGHT
- CLASS "B" LAP, TYPICAL
- 2. CLASS "A" SPLICES ARE ALLOWED WHEN ONE HALF OF THE TOTAL BARS ARE SPLICED
- WITHIN THE REQUIRED LAP LENGTH OF THE BARS.
- 3. CLASS "A" SPLICE = CLASS "B" SPLICE / 1.3

REV. DATE DESCRIPTION

1 7/2015 75% SUBMITTAL

0 5/2015 50% SUBMITTAL

TANK FARM FOUNDATION NOTES



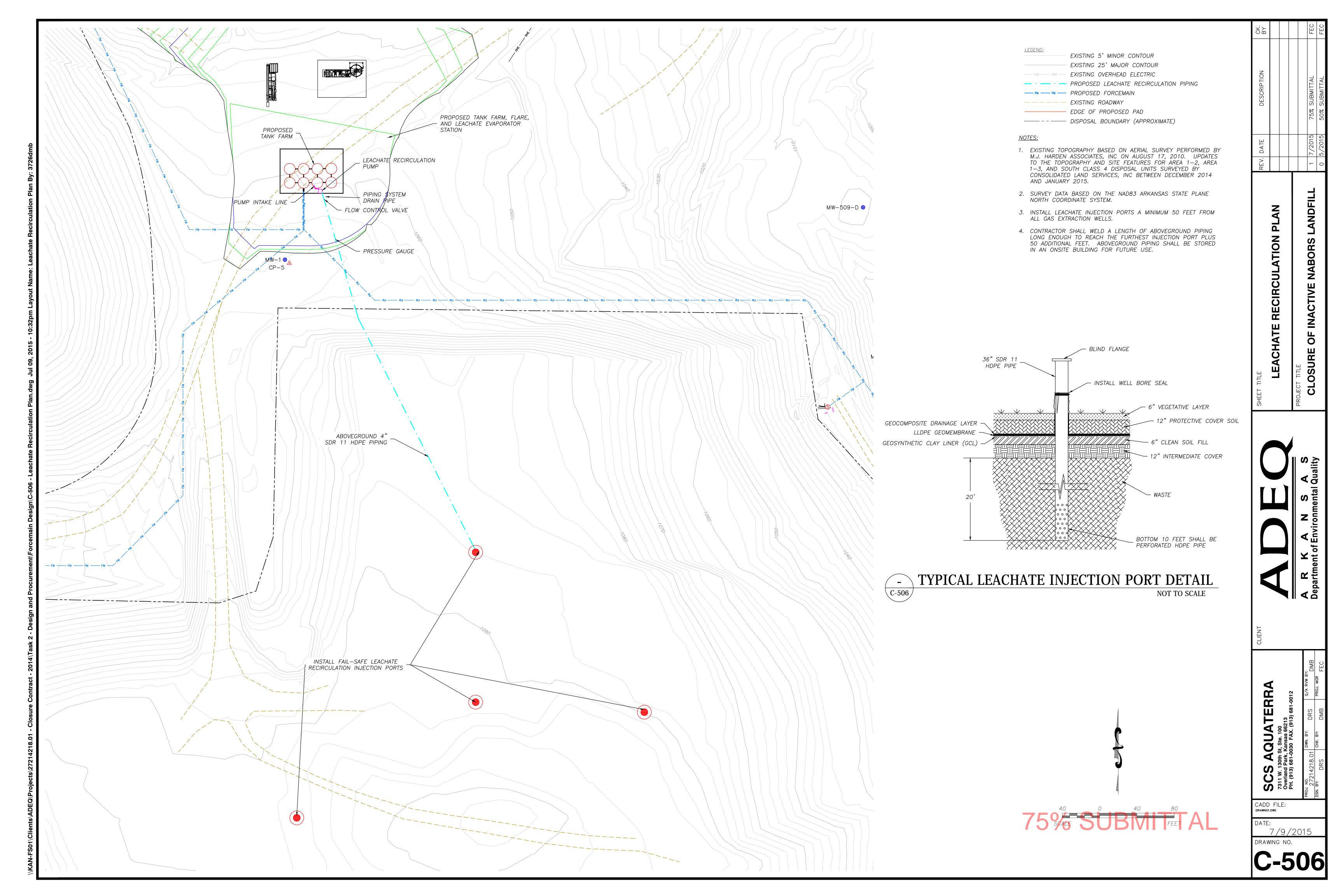
SCS AQUATERRA
7311 W. 130th St, Ste. 100
Overland Park, Kansas 66213
PH. (913) 681-0030 FAX. (913) 681-0012

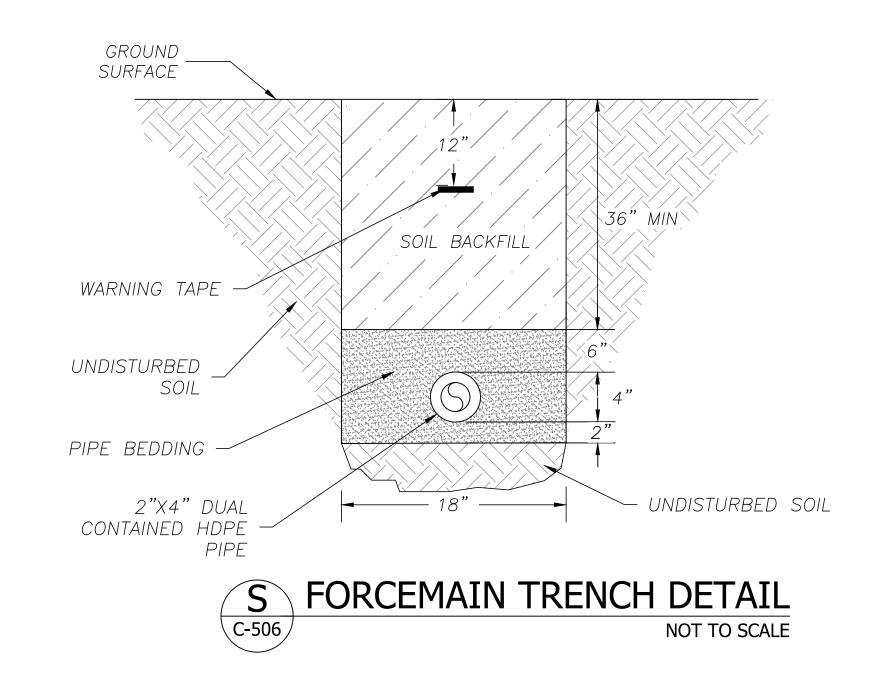
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COJ. NO.
CO

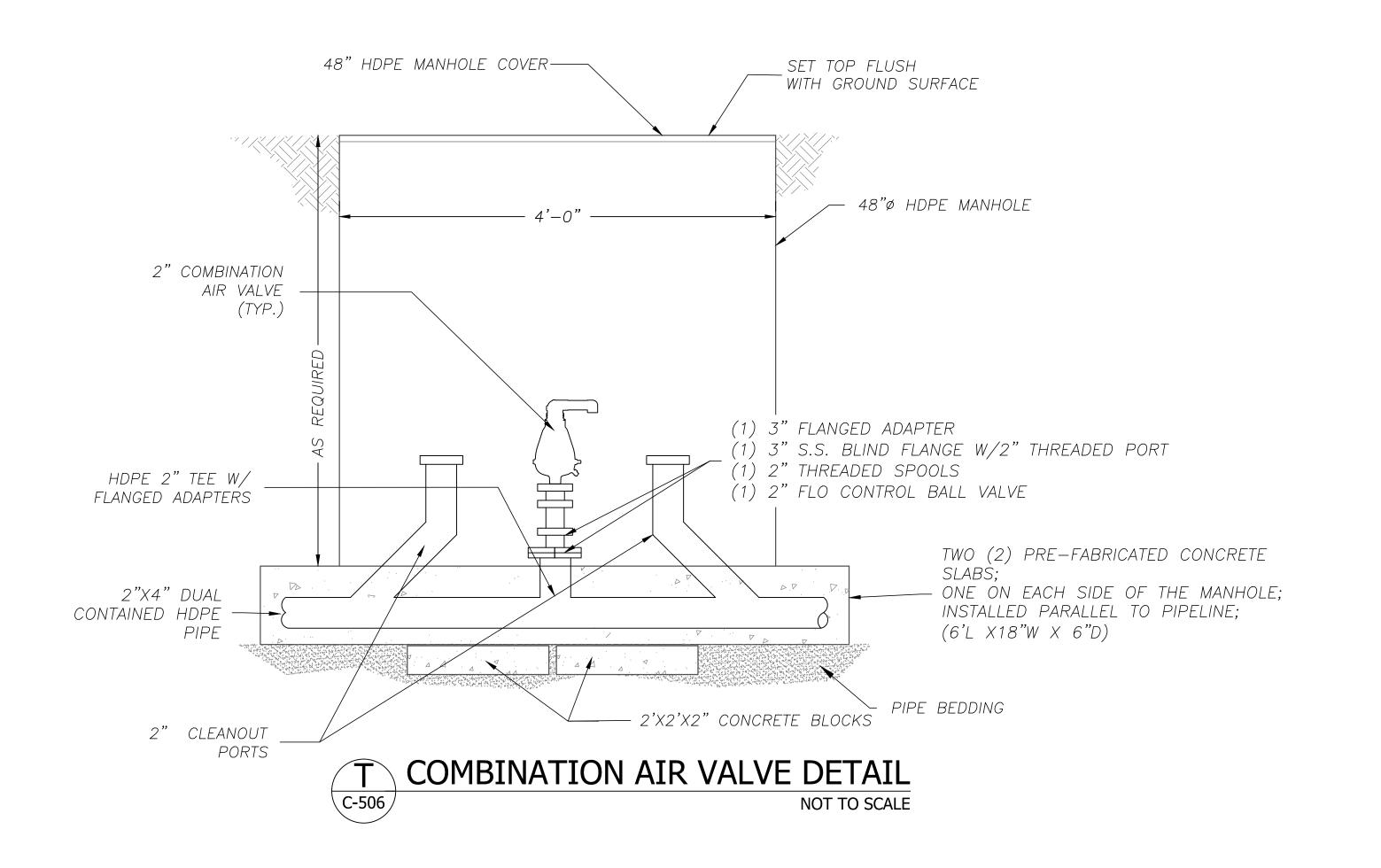
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7/8/15
DRAWING NO.

75% SUBMITTAL







LEACHATE DISPOSAL SYSTEM D

A R K A N S A S
Department of Environmental Quality

CLOSURE OF INACTIVE NABORS LA

AQUATERRA 130th St. Ste. 100

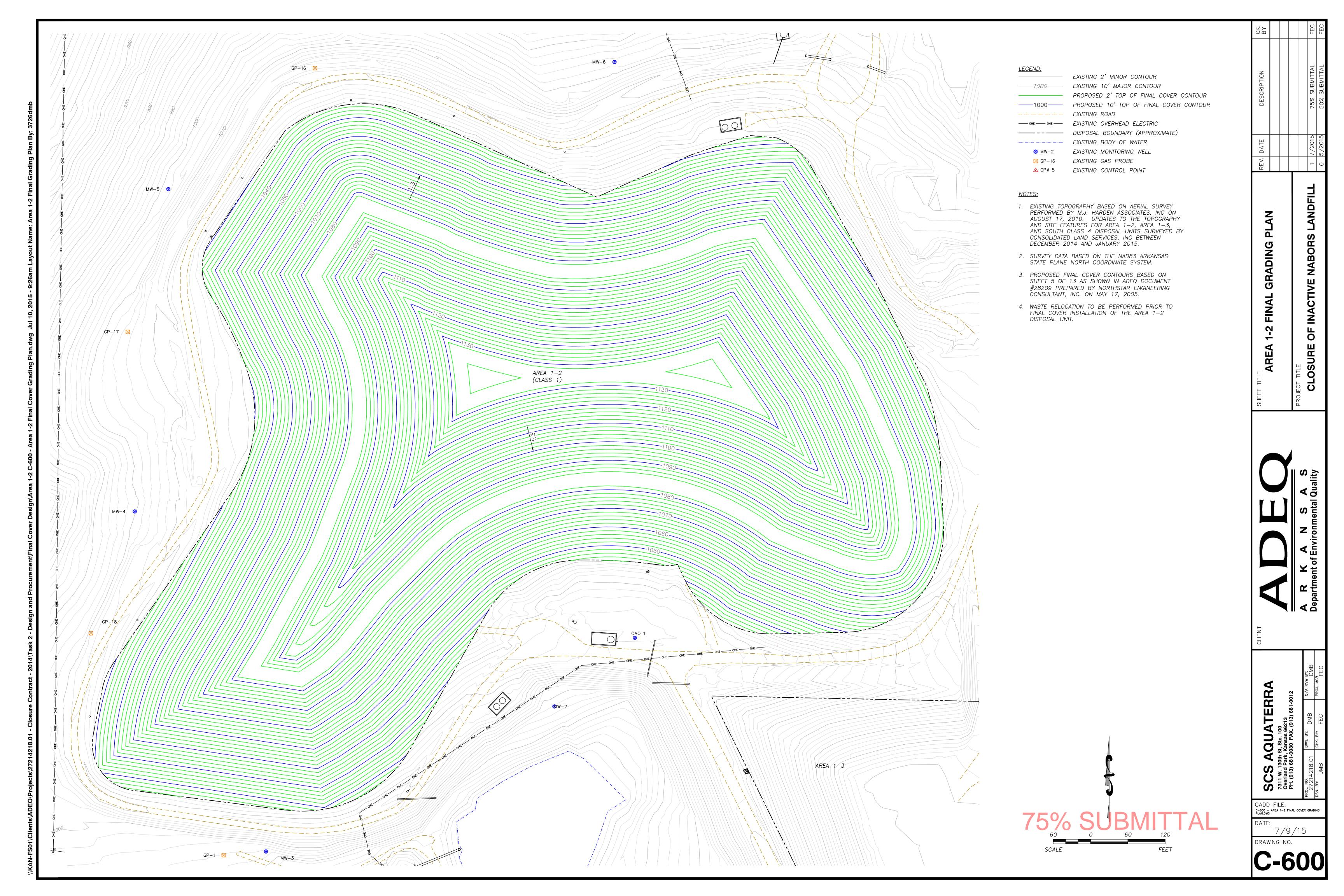
**SCS**7311 W. 13
Overland P
PH. (913) 6

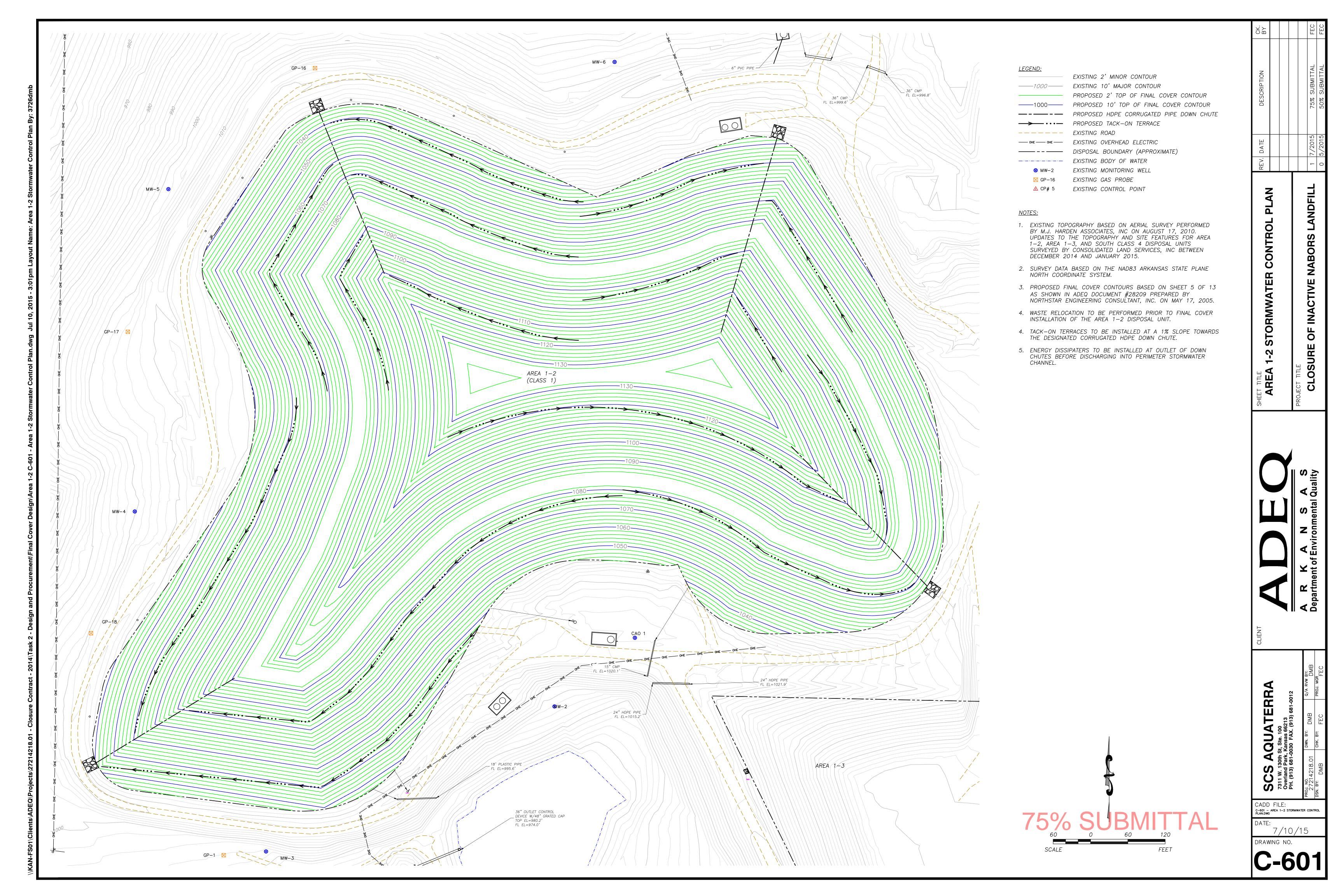
DATE: 7/9/2015

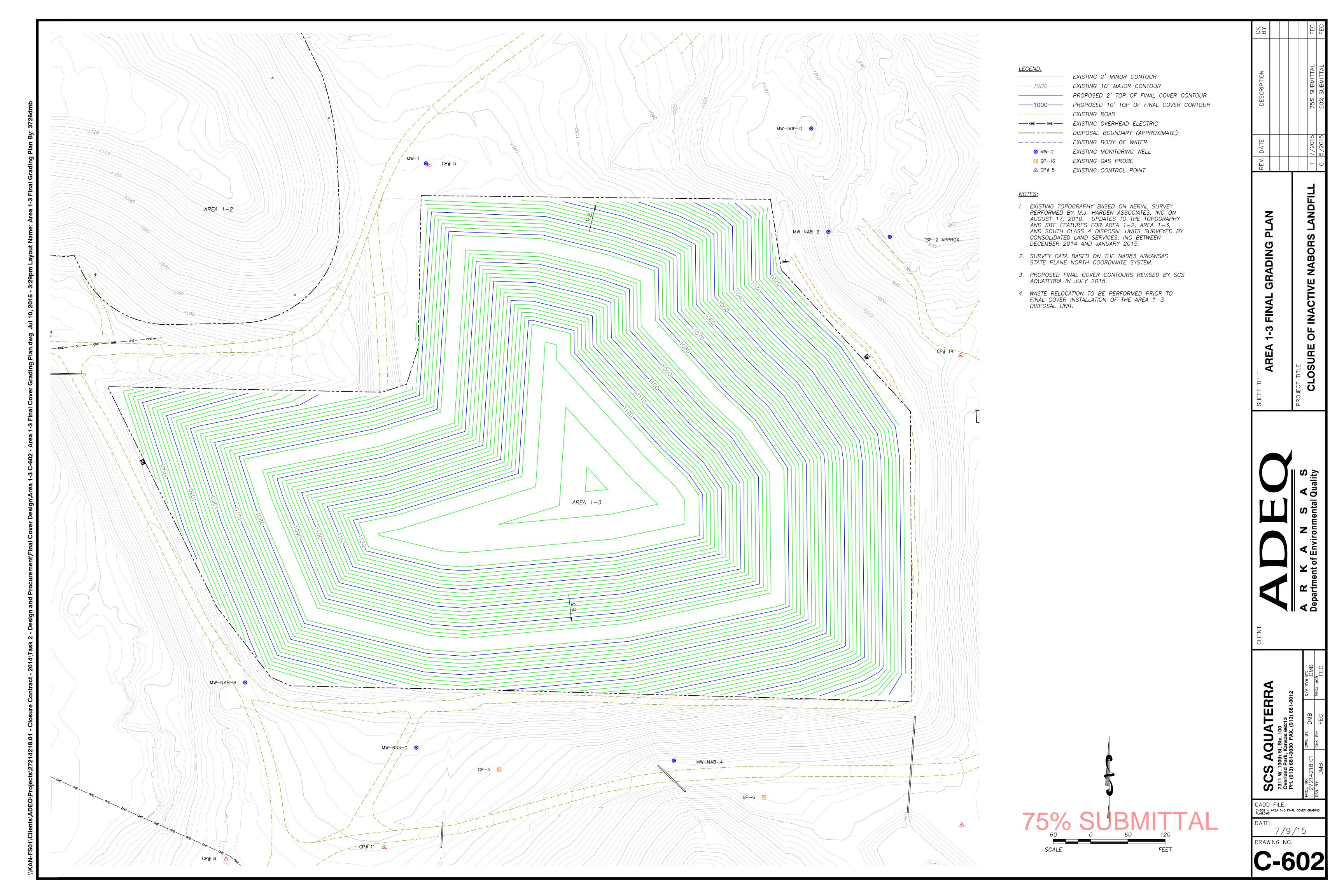
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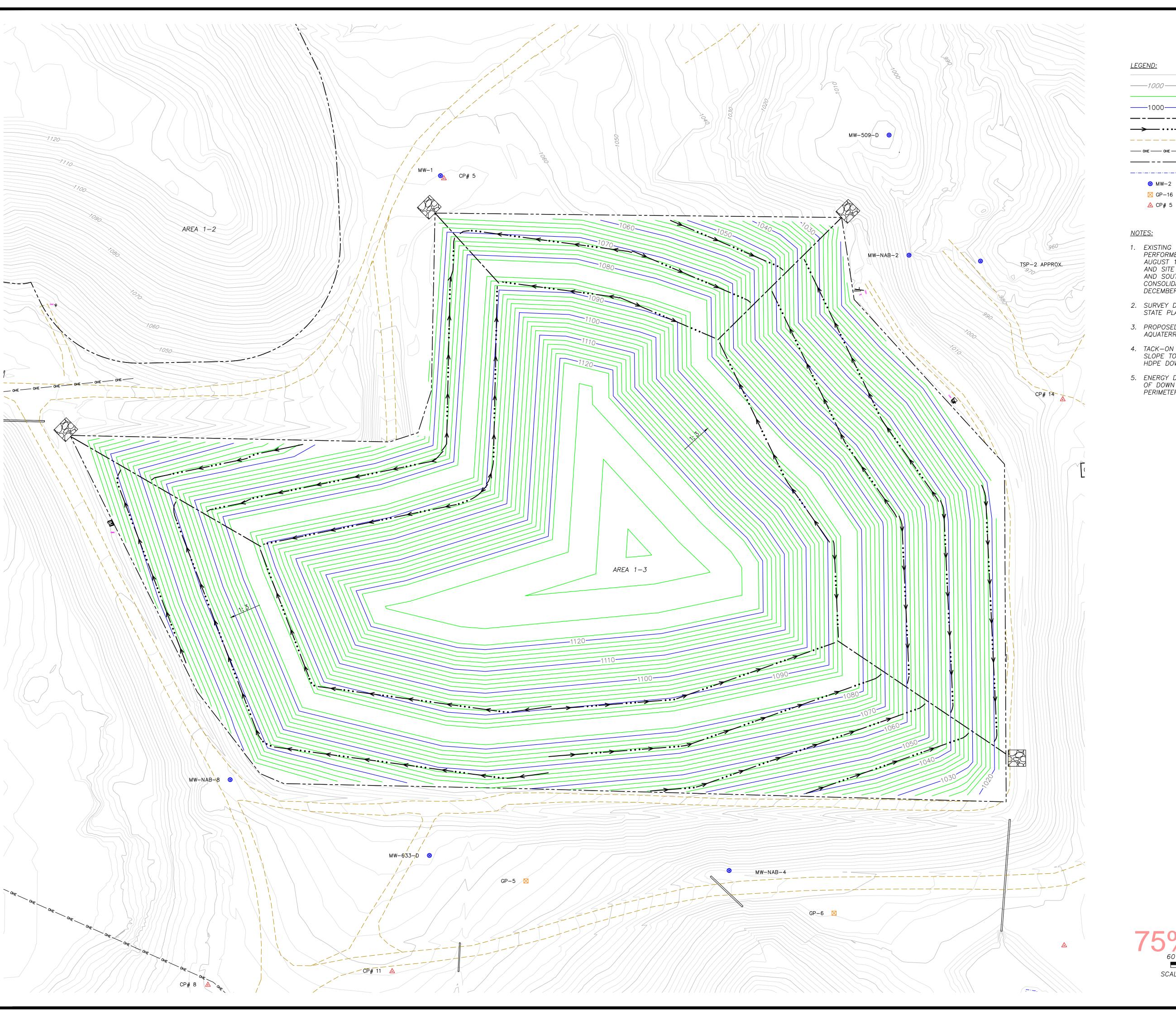
DRAWING NO.

75% SUBMITTAL









EXISTING 2' MINOR CONTOUR

----- EXISTING BODY OF WATER

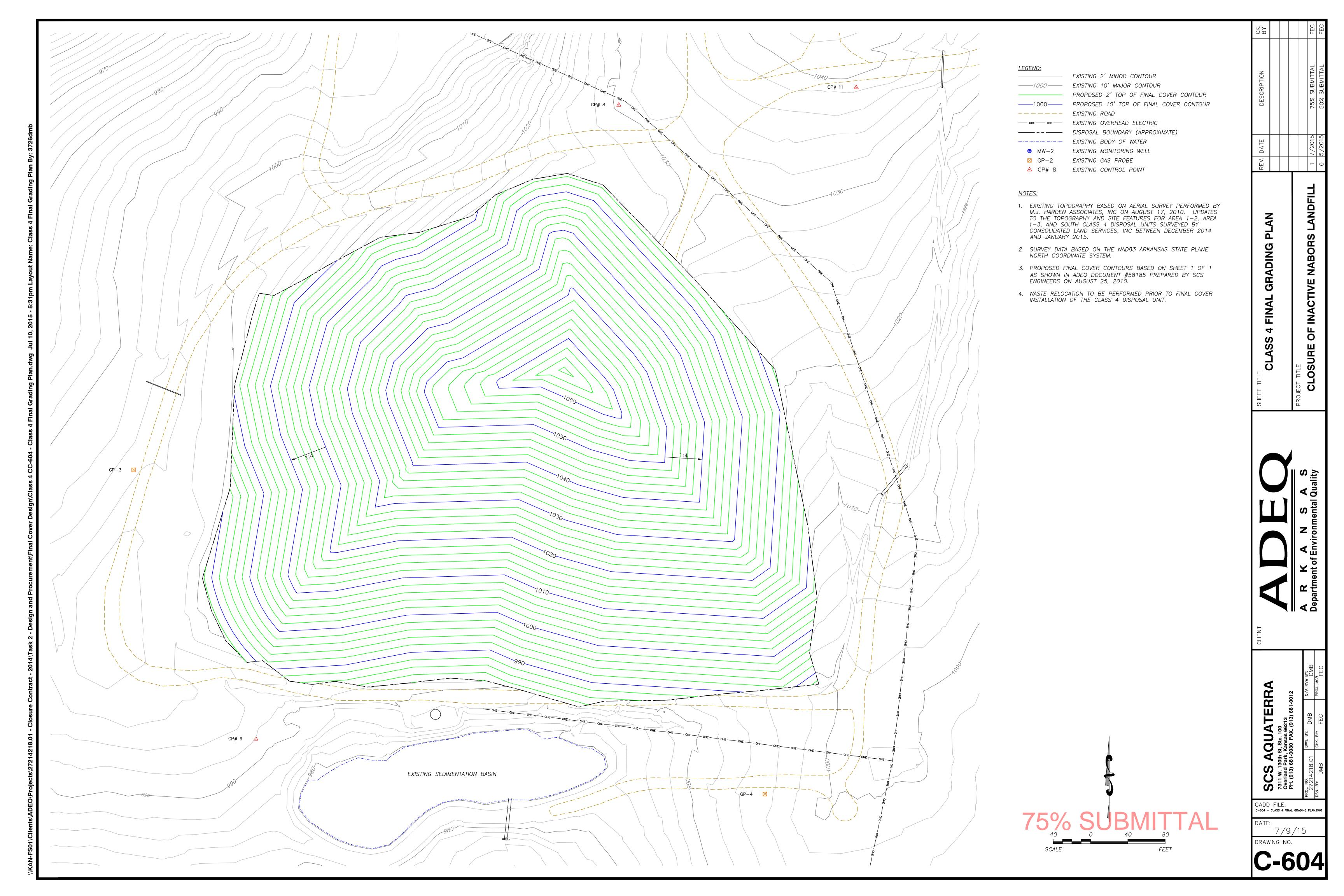
MW-2 EXISTING MONITORING WELL

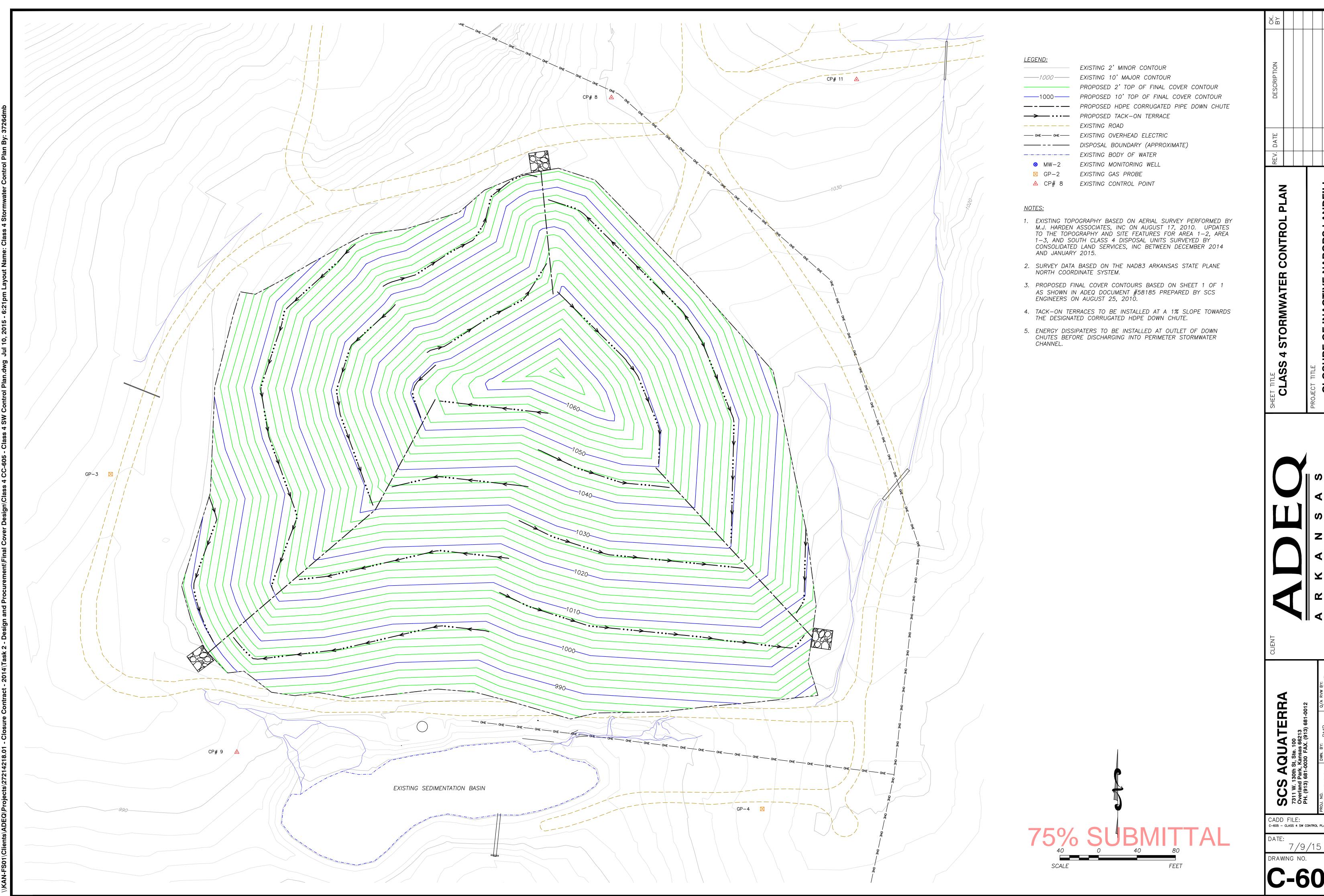
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- 2. SURVEY DATA BASED ON THE NAD83 ARKANSAS STATE PLANE NORTH COORDINATE SYSTEM.
- 3. PROPOSED FINAL COVER CONTOURS REVISED BY SCS AQUATERRA IN JULY 2015.
- 4. TACK—ON TERRACES TO BE INSTALLED AT A 1% SLOPE TOWARDS THE DESIGNATED CORRUGATED HDPE DOWN CHUTE.
- 5. ENERGY DISSIPATERS TO BE INSTALLED AT OUTLET OF DOWN CHUTES BEFORE DISCHARGING INTO PERIMETER STORMWATER CHANNEL.

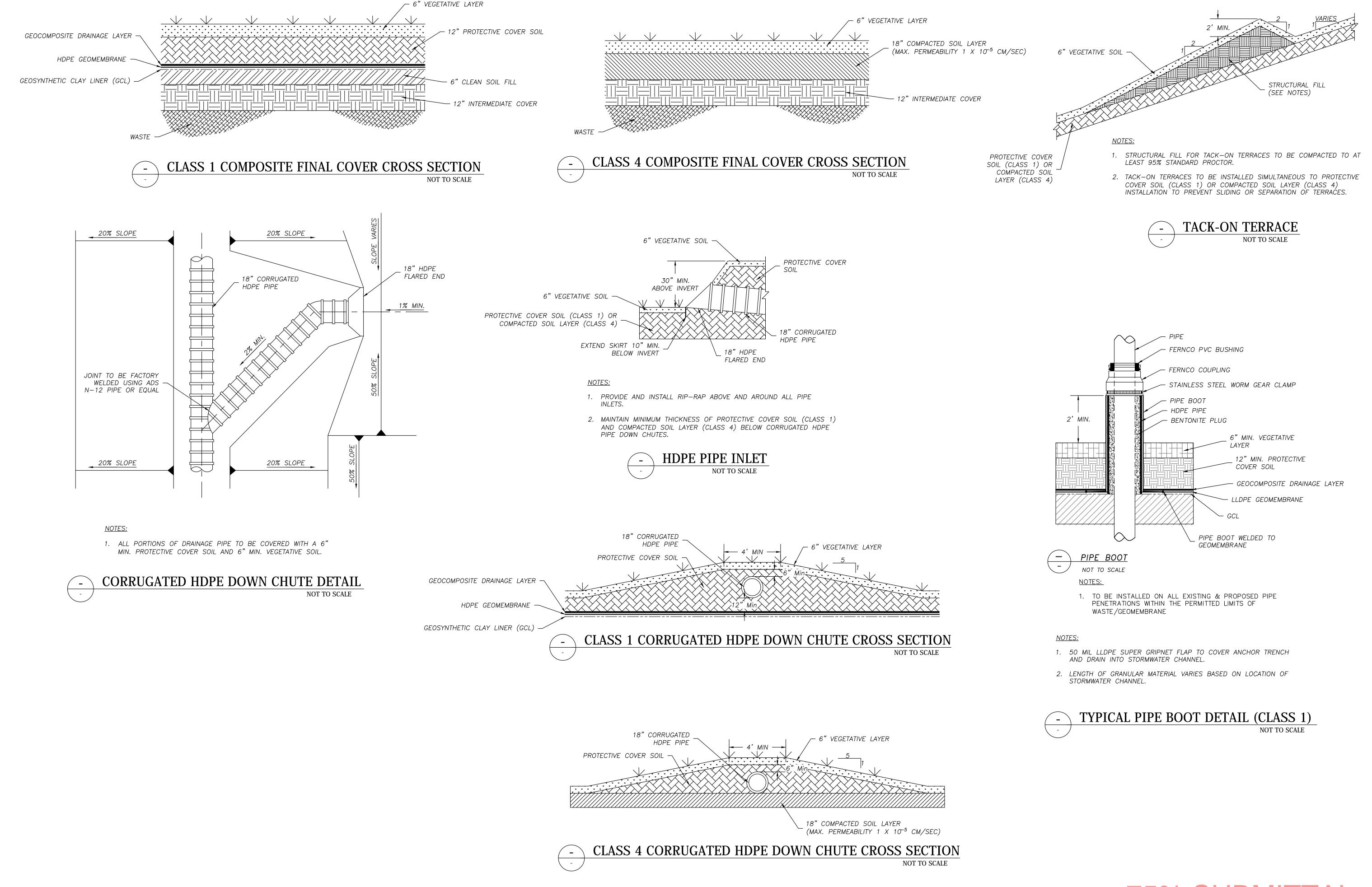
SCS AQUATERRA
7311 W. 130th St, Ste. 100
Overland Park, Kansas 66213
PH. (913) 681-0030 FAX. (913) 681-0012
PROJ. NO. NO. DWN. BY: DMB Q/A RWW

ADD FILE:
603 – AREA 1–3 STORMWATER CON
ATF:

7/10/15 DRAWING NO.







75% SUBMITTAL

ATERR

CADD FILE: C-606 - FINAL COVER DETAILS.DWG

7/10/15

